ABBREVIATIONS

A.B.	ANCHOR BOLT	HK.	HOOK
ACI	AMERICAN CONCRETE INSTITUTE	HORIZ.	HORIZONTAL
		H.P.	HIGH POINT
ADJMT.	ADJUSTMENT	I.E.	INVERT ELEVATION
ADDTL.	ADDITIONAL	I.F.	INSIDE FACE
ANCH.	ANCHOR	INCL.	INCLUSIVE
ARCH.	ARCHITECTURAL	INFO.	INFORMATION
ASTM	AMERICAN SOCIETY FOR TESTING AND MATERIALS	INS.	INSULATION
		JST.	JOIST
BLDG.	BUILDING	JT.	JOINT
BLKG.	BLOCKING	K.	KIPS (1000 LB.)
BM.	BEAM	KSF	KIPS PER SQUARE FOOT
BOT.	BOTTOM	KSI	KIPS PER SQUARE INCH
BPL.	BASEPLATE	L.L.	LIVE LOAD
BRCG.	BRACING	LLH	LONG LEG HORIZONTAL
BRG.	BEARING	LLV	LONG LEG VERTICAL
BRK.	BRICK	L.P.	LOW POINT
BRKT.	BRACKET	LWT.	LIGHT WEIGHT
B.S.	BOTH SIDE (USED W/ REINF)	MAS.	MASONRY
B.S.	BRICK SHELF	MAX.	MAXIMUM
BSMT.	BASEMENT	MECH.	MECHANICAL
BTWN.	BETWEEN	MTL.	METAL
B.W.	BOTH WAYS	MFR.	MANUFACTURER
CANT.	CANTILEVER	MIN.	MINIMUM
C.B.	CONCRETE BEAM	NO.	NUMBER
CFMF	COLD FORMED METAL FRAMING	N.S.	NEAR SIDE
C.J.	CONTROL JOINT	N–S	NORTH–SOUTH
CL.	CENTERLINE	NWT.	NORMAL WEIGHT
CLR.	CLEAR	O.C.	ON CENTER
CMU	CONCRETE MASONRY UNIT	O.F.	OUTSIDE FACE
COL.	COLUMN	OPNG.	OPENING
COMP.	COMPRESSIBLE	OPP.HD.	OPPOSITE HAND
CONC.	CONCRETE	ORIENT.	ORIENTATION
CONN.	CONNECTION	PC.	PRECAST
CONST.	CONSTRUCTION	P.C.	PRECAST CONCRETE
CONT.	CONTINUOUS	PCF	POUNDS PER CUBIC FOOT
CONTR.	CONTRACTOR	PED.	PEDESTAL
COORD.	COORDINATE	PENET.	PENETRATION
CR.	CRIPPLED	PL.	PLATE
DBL.	DOUBLE	PLUMB.	PLUMBING
DET.	DETAIL	PLWD.	PLYWOOD
DEVELOP.	DEVELOPMENT	PNL.	PANEL
DIAG.	DIAGONAL	PREM.	PREMOLDED
DIST.	DISTANCE	PSI	POUNDS PER SQUARE INCH
DK.	DECK	PSF	POUNDS PER SQUARE FOOT
D.L.	DEAD LOAD	REINF.	REINFORCING
DN.	DOWN	REQD.	REQUIRED
DO	DITTO	RET.	RETAINING
DWG.	DRAWING	REV.	REVISION
DWL.	DOWEL	RF.	ROOF
EA.	EACH	R.O.	ROUGH OPENING
E.F.	EACH FACE	SCHED.	SCHEDULE
EL.	ELEVATION	SECT.	SECTION
ELEC.	ELECTRICAL	SL.	SLOTTED
ELEV.	ELEVATOR	SPAN.	SPANDREL
EMB.	EMBED, EMBEDMENT	SPEC.	SPECIFICATIONS
EQ.	EQUAL	STAGG.	STAGGERED
ETC.	ETCETERA	STD.	STANDARD
E.W.	EACH WAY	STIFF.	STIFFENER
E–W	EAST–WEST	STL.	STEEL
EXIST.	EXISTING	SUPP.	SUPPORT
EXP.	EXPANSION	T.&B.	TOP AND BOTTOM
EXP. JT.	EXPANSION JOINT	TEMP.	TEMPORARY
FDN.	FOUNDATION	THK.	THICK, THICKNESS
FIN.	FINISH	T.O.C.	TOP OF CONCRETE
FLG.	FLANGE	TOL.	TOLERANCE
FLR.	FLOOR	T.O.P.	TOP OF PEDESTAL
FRMG.	FRAMING	T.O.S.	TOP OF STEEL
F.S.	FAR SIDE	T.O.W.	TOP OF WALL
FTG.	FOOTING	TYP.	TYPICAL
GA.	GAGE	U.N.O.	UNLESS NOTED OTHERWISE
GALV.	GALVANIZED	V.	VERTICAL (USED W/ REINF.)
GB	GRADE BEAM	VERT.	VERTICAL
G.C.	GENERAL CONTRACTOR	V.I.F.	VERIFY IN FIELD
GRAN.	GRANULAR	WD.	WOOD
GR. BM.	GRADE BEAM	W.P.	WORK POINT
H.	HORIZONTAL (USED W/ REINF.)	W.S.	WATER STOP
		WWF	WELDED WIRE FABRIC

GENERAL NOTES

GENERAL NOTES

1. THE GENERAL CONTRACTOR SHALL VERIFY ALL EXISTING CONDITIONS AND DIMENSIONS AT THE SITE BEFORE ORDERING ANY MATERIALS AND BEGINNING ANY WORK, THE GENERAL CONTRACTOR SHALL FIELD SURVEY AND ESTABLISH THE EXISTING BUILDING DIMENSIONS WHERE NEW CONSTRUCTION ABUTS EXISTING BUILDINGS. THIS FIELD SURVEY SHALL INCLUDE, BUT SHALL NOT BE LIMITED TO THE FOLLOWING: DIMENSIONS OF EXISTING BUILDING FACE INCLUDING ALL FENESTRATIONS, PROJECTIONS, ETC, PLUMBNESS OF WALLS, FLOOR AND ROOF ELEVATIONS, AND ALL OTHER PERTINENT DIMENSIONS. THIS FIELD SURVEY SHALL BE FOR THE USE BY ALL CONTRACTORS AND SHALL BE SUBMITTED TO THE OWNER AND ENGINEER FOR RECORD ONLY.
2. THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR ALL WORK AND COORDINATION INVOLVED TO PROVIDE ALL OPENINGS SHOWN ON THE ARCHITECTURAL, STRUCTURAL, MECHANICAL, PLUMBING, AND ELECTRICAL DRAWINGS. GENERAL CONTRACTOR SHALL PROVIDE FRAMING AND ALL CONNECTIONS AND COORDINATE THE SIZE AND LOCATION OF ALL OPENINGS. (NOTE – NOT ALL OPENINGS ARE SHOWN ON THE STRUCTURAL DRAWINGS.)
3. ALL CONTRACTORS SHALL BE RESPONSIBLE TO ENSURE PROPER STORAGE OF MATERIAL IS MAINTAINED SO AS NOT TO CAUSE OVERLOADING OF THE EXISTING OR NEW STRUCTURE DURING PERFORMANCE OF THIS WORK. GENERAL CONTRACTOR TO COORDINATE.
4. ALL CONTRACTORS SHALL VERIFY AND/OR ESTABLISH ALL EXISTING CONDITIONS AND DIMENSIONS AT THE SITE BEFORE ORDERING ANY MATERIAL AND COMMENCEMENT OF ANY WORK.
5. IF THE EXISTING CONDITIONS DO NOT PERMIT THE INSTALLATION OF THE WORK IN ACCORDANCE WITH THE DETAILS AS SHOWN, THE CONTRACTOR SHALL NOTIFY THE ENGINEER IMMEDIATELY AND PROVIDE A SKETCH OF THE CONDITION WITH HIS PROPOSED MODIFICATION TO THE DETAILS GIVEN ON THE CONTRACT DOCUMENTS. THE FINAL INSTALLATION SHALL BE DONE AS REQUIRED BY THE ENGINEER, AT NO ADDITIONAL COST TO THE OWNER.
6. WHERE ALTERATIONS INVOLVE THE EXISTING SUPPORTING STRUCTURE, THE CONTRACTOR SHALL PROVIDE ALL SHORING, BRACING, GUYS AND PROTECTION REQUIRED TO ENSURE THE STRUCTURAL INTEGRITY OF THE EXISTING BUILDING.
7. ALL STRUCTURAL STEEL ANGLES ATTACHED TO THE STRUCTURAL STEEL TO SUPPORT THE ARCHITECTURAL BUILDING SKIN MATERIALS (PRECAST PANEL SYSTEM, METAL PANEL SYSTEM WINDOW WALL SYSTEM, SKYLIGHT SYSTEM, AND MASONRY, ETC.) NOT PART OF EACH WALL SYSTEM SHOWN ON STRUCTURAL AND/OR ARCHITECTURAL DRAWINGS ARE TO BE PROVIDED UNDER THE METAL FABRICATIONS SECTION 05500 OF THE SPECIFICATIONS. CONTRACTOR MUST COORDINATE DETAILS SHOWN ON STRUCTURAL DRAWINGS WITH ARCHITECTURAL DRAWINGS. GENERAL CONTRACTOR SHALL BE RESPONSIBLE TO ASSIGN WHO FURNISHES AND INSTALLS ALL SUCH SUPPORTING ANGLES SHOWN ON THE DRAWINGS AND REQUIRED BY THE RESPECTIVE SUBCONTRACTORS AND/OR TRADES.
8. THE CONTRACTOR SHALL VERIFY ALL OPENINGS SHOWN ON THE STRUCTURAL DRAWINGS WITH WITH THE DIMENSIONS AND LOCATIONS SHOWN ON THE ARCHITECTURAL DRAWINGS AS WELL AS DRAWINGS OF OTHER TRADES PRIOR TO CONSTRUCTION.
9. THE CONTRACTOR TO COORDINATE ALL RELATED TRADE ACTIVITY REGARDING SHUT DOWNS, RE–ROUTING, TEMPORARY INSTALLATION, ETC. NECESSARY FOR THIS INSTALLATION WITH OWNER'S REPRESENTATIVE.
10. THE GENERAL CONTRACTOR SHALL ESTABLISH SPECIFIC MEANS AND METHODS FOR INSTALLATION AND SHALL COORDINATE THE WORK FOR ALL CONTRACTORS AND COMPLY WITH OWNER'S REQUIREMENTS.

FOUNDATION NOTES

1. ALL FOOTINGS SHALL BEAR ON UNDISTURBED STRATUM HAVING A MINIMUM ALLOWABLE BEARING PRESSURE OF 2000 PSF, VERIFIED IN THE FIELD BY A GEOTECHNICAL ENGINEER HIRED BY THE OWNER, UNLESS OTHERWISE NOTED.
2. ALL COLUMN FOOTINGS SHALL BE CENTERED ON THE COLUMN CENTERLINES, UNLESS OTHERWISE NOTED.
3. THE CONTRACTOR SHALL PROVIDE ALL DEWATERING AS REQUIRED DURING THE EXCAVATION AND CONSTRUCTION OF THE FOUNDATION WORK INCLUDING PREVENTIVE MEASURES RELATED TO EXCAVATION STABILITY, SEE SPECIFICATIONS.
4. BOTTOM OF NEW FOOTING ELEVATION INDICATED THUS (...) IN PLAN. BOTTOM OF EXISTING FOOTING ELEVATION INDICATED THUS (±...) IN PLAN. SEE FOUNDATION NOTES ON DRAWING S2.1.
5. BOTTOM OF FOOTING GIVEN IN THE FOUNDATION PLAN MARKED THUS (...) ARE APPROXIMATE AND MUST BE VERIFIED IN THE FIELD IN ACCORDANCE WITH NOTES ABOVE.
6. ALL EXISTING UNDERGROUND UTILITIES IN THE AREA OF THE NEW CONSTRUCTION SHALL BE RELOCATED UNLESS OTHERWISE NOTED ON THE DRAWINGS BEFORE ANY NEW FOUNDATION WORK IS STARTED. EXISTING SITE ELEMENTS AND UTILITIES, MANHOLES, CATCH BASINS, ETC. ADJACENT TO NEW CONSTRUCTION EXCAVATIONS SHALL BE PROTECTED BY SHEETING AND/OR SHORING. THIS PROTECTION SHALL BE PROVIDED AND DESIGNED BY THE GENERAL CONTRACTOR AND HIS REGISTERED PROFESSIONAL ENGINEER, LICENSED IN THE DISTRICT OF COLUMBIA WHO SHALL BE TOTALLY RESPONSIBLE FOR ITS DESIGN AND INSTALLATION.
7. THE CONTRACTOR SHALL COORDINATE ALL FOUNDATION WORK WITH ALL UNDERGROUND UTILITIES. ALL NEW UNDERGROUND UTILITIES OR PIPES SHALL NOT BE PLACED BELOW SPREAD FOOTINGS. IF ANY SUCH CONDITION OCCURS, THE CONTRACTOR SHALL NOTIFY THE ENGINEER AND DROP THE BOTTOM OF FOOTING TO CLEAR THE PIPE AT NO ADDITIONAL COST TO THE OWNER.
8. ALL UNDERGROUND UTILITIES, SITE ELEMENTS, MANHOLES, CATCH BASINS, ETC. ADJACENT TO NEW CONSTRUCTION EXCAVATIONS SHALL BE PROTECTED BY SHEETING AND/OR SHORING. THIS PROTECTION SHALL BE PROVIDED BY THE CONTRACTOR WHO SHALL BE TOTALLY RESPONSIBLE FOR ITS DESIGN AND INSTALLATION.
9. CONTRACTOR SHALL COORDINATE ALL FOUNDATION WORK WITH ALL UNDERGROUND UTILITIES. EXTREME CARE SHALL BE TAKEN DURING EXCAVATION AND CONSTRUCTION OF NEW FOUNDATION WORK SO AS NOT TO DISTURB THE EXISTING CONSTRUCTION AND UTILITIES.
10. PROVIDE STANDARD STEEL PIPE SLEEVES FOR ALL PIPES PASSING THROUGH NEW CONCRETE WALLS AND NEATLY CORED HOLES A MINIMUM OF ONE PIPE SIZE LARGER THAN NEW PIPE THROUGH EXISTING CONCRETE WALLS WHERE SHOWN ON THE DRAWINGS. COORDINATE CORED HOLES WITH SEALANT, ETC., REQUIREMENTS WITH RELATED SPECIFICATIONS. SEE TYPICAL DETAIL ON DRAWING S5.1.
11. WHERE THE EXCAVATION FOR SERVICE LINE TRENCHES IS LOWER THAN AND CLOSER THAN A 1.5H:1V SLOPE TO THE BOTTOM OF A NEW OR EXISTING COLUMN OR WALL FOOTING, BACKFILL THE EXCAVATION WITH LEAN MIX CONCRETE. TOP OF FILL TO BE ON A 1.5H:1V SLOPE FROM BOTTOM OF ADJACENT FOUNDATIONS.
12. THE TEST BORINGS FOR THIS PROJECT WERE PERFORMED BY:
GEOCONCEPTS ENGINEERING, INC.
19955 HIGHLAND VISTA DR.
SUITE 170
ASHBURN, VA 20147
A COPY OF THE SOILS AND FOUNDATION INVESTIGATION ANALYSIS REPORT IS INCLUDED IN THE SPECIFICATION FOR INFORMATION ONLY.
13. FOR ADDITIONAL REQUIREMENTS SEE TYPICAL DETAILS AND THE SPECIFICATIONS.

CONCRETE NOTES

1. ALL CONCRETE SLABS ON GRADE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 4000 PSI AT 28 DAYS. ALL REINFORCING TO BE ASTM A615 GRADE 60. ALL MESH SHALL BE ASTM A185.
2. ALL OTHER CONCRETE INCLUDING FOUNDATIONS, WALLS, PIERS, STRUCTURAL SLABS, BEAMS, PEDESTALS, EQUIPMENT PADS, SIDEWALKS, ETC. SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 4000 PSI AT 28 DAYS. ALL REINFORCING STEEL SHALL BE A615, GRADE 60.
3. ALL LIGHT WEIGHT CONCRETE SLAB ON METAL DECK SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS. ALL MESH SHALL BE ASTM A185. ALL REINFORCING TO BE ASTM A615 GRADE 60.
4. ALL CONCRETE WORK SHALL BE CURED FOR A MINIMUM OF 7 DAYS IN ACCORDANCE WITH ACI STANDARDS.
5. CONTRACTOR SHALL VERIFY THE DIMENSIONS OF AND INSTALL IN THE FORMS ALL SLOTS, SLEEVES, ANCHOR BOLTS, MASONRY ANCHORS, POCKETS, ETC. AS REQUIRED FOR OTHER TRADES.
6. SEE MECHANICAL AND ELECTRICAL DRAWINGS FOR CONCRETE EQUIPMENT PADS AND FOUNDATIONS REQUIRED.
7. ALL CONCRETE USED TO PATCH EXISTING FLOOR SLABS SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 4000 PSI AT 28 DAYS. THE CONCRETE TYPE (NORMAL WEIGHT OR LIGHTWEIGHT) SHALL BE THE SAME AS THE EXISTING ADJACENT CONCRETE.
8. ALL SHORING AND/OR RESHORING FOR SUPPORTED CONCRETE SLABS SHALL BE LEFT IN PLACE UNTIL THE CONCRETE HAS REACHED ITS 28 DAY STRENGTH AND A MINIMUM OF 14 DAYS.
9. SEE SECTIONS AND DETAILS FOR ALL EQUIPMENT OPENINGS, DEPRESSIONS, ETC. CONTRACTOR SHALL COORDINATE EQUIPMENT REQUIREMENTS WITH ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING CONTRACTORS.
10. EXPANSION FASTENERS (BOLTS) INSTALLED TO EXISTING OR NEW CONCRETE CONSTRUCTION SHALL NOT BE INSTALLED CLOSER THAN 4 INCHES TO THE EDGE OF THE CONCRETE, AND MUST AVOID ANY REINFORCING.
11. FOR ADDITIONAL REQUIREMENTS, SEE TYPICAL DETAILS AND THE SPECIFICATIONS.

STRUCTURAL STEEL NOTES

1. ALL STRUCTURAL STEEL WIDE FLANGE MEMBERS TO BE ASTM A992 GRADE 50. ALL STRUCTURAL STEEL BASE PLATES, MOMENT PLATES AND SPLICE PLATES TO BE ASTM A572 GRADE 50. ALL HOLLOW STRUCTURAL STEEL MEMBERS SHALL BE ASTM A500 GRADE B. ALL STRUCTURAL STEEL ANGLES, CHANNELS AND OTHER PLATES TO BE A36.
2. THE STRUCTURAL STEEL CONTRACTOR SHALL VERIFY IN THE FIELD BY A SURVEY ALL EXISTING CONDITIONS CONNECTED WITH HIS WORK INCLUDING ANCHOR BOLT LOCATIONS PRIOR TO ORDERING ANY MATERIAL OR COMMENCEMENT OF ANY WORK.
3. THE STRUCTURAL STEEL CONTRACTOR SHALL PROVIDE SATISFACTORY BRACING OF THE EXISTING AND NEW STEEL FRAME UNTIL ALL NEW FRAMING AND THE METAL DECK IS ERECTED AND FINAL CONNECTIONS ARE COMPLETE AND THE CONCRETE SLABS ON METAL DECK ARE PLACED.
4. ALL STRUCTURAL STEEL MEMBERS, I.E. SHELF ANGLES, CHANNELS, ETC. WHICH DIRECTLY SUPPORT THE ARCHITECTURAL BUILDING SKIN SHALL BE FABRICATED AND ERECTED TO WITHIN 3/16" OF THE THEORETICAL SUPPORT POSITION SHOWN ON THE CONTRACT DOCUMENTS. ALL SUCH MEMBERS WHICH BUTT SHALL HAVE THE SAME POSITION AT THE BUTT LINE TO ENSURE A CONTINUOUS SURFACE FOR SUPPORT ACROSS THE BUTT LINE.
5. ALL SHIMS USED IN POSITIONING THE STRUCTURAL STEEL FOR SUPPORTING THE ARCHITECTURAL BUILDING SKIN SHALL BE FULL BEARING STEEL FINGER SHIMS AND UPON FINAL ALIGNMENT ALL SUCH SHIMS SHALL BE TACK WELDED TOGETHER AS WELL AS TO THE CONFINING STEEL TOP AND BOTTOM.
6. MAIN SUPPORT MEMBERS FOR THE METAL DECK ARE SHOWN ON THE CONTRACT DRAWINGS. DURING PREPARATION, SUBMISSION, AND REVIEW OF SHOP DRAWINGS ANY ADDITIONAL SUPPORT OR ATTACHMENT DETAILS REQUIRED TO ESTABLISH THE METAL DECK AT THE REQUIRED ELEVATION SHALL BE PROVIDED BY THE STRUCTURAL STEEL CONTRACTOR AT NO ADDITIONAL COST.
7. BEAM TO BEAM AND/OR BEAM TO COLUMN CONNECTIONS MARKED (MC) SHALL BE DETAILED TO DEVELOP FULL MOMENT CAPACITY AT THE CONNECTION IN ADDITION TO STANDARD SHEAR CONNECTION. THESE MOMENT CONNECTIONS ARE TO BE MADE BY FULL PENETRATION WELDS OF BOTH BEAM FLANGES. COORDINATE THESE DETAILS WITH OTHER FRAMING ELEMENTS AS REQUIRED.
8. STRUCTURAL STEEL ERECTOR: NOTE THAT SEQUENCE OF ERECTION TO BE COORDINATED AS REQUIRED FOR AREAS SUPPORTED BY CANTILEVERS. ALL MOMENT CONNECTIONS AND/OR OTHER CONNECTIONS FOR CANTILEVERED FRAMING SHALL HAVE TEMPORARY BRACING AND SUPPORT OF CANTILEVER FRAMING UNTIL ALL FINAL CONNECTIONS ARE COMPLETED AND INSPECTED BY THE TESTING AND INSPECTION AGENCY, AND THE RESULTS ACCEPTED PRIOR TO ERECTING FRAMING SUPPORTED BY THE CANTILEVER ENDS.
9. FOR ADDITIONAL REQUIREMENTS, SEE TYPICAL DETAILS AND THE SPECIFICATIONS.

METAL DECK NOTES

1. ALL METAL DECK SHALL CONFORM TO THE REQUIREMENTS OF THE STEEL DECK INSTITUTE (SDI).
2. ALL COMPOSITE METAL FLOOR DECK SUPPORTING LIGHT WEIGHT CONCRETE SLAB SHALL BE CONTINUOUS OVER A MINIMUM OF TWO OR MORE SPANS.
3. FOR ADDITIONAL INFORMATION SEE THE SPECIFICATIONS.

COLD FORMED METAL FRAMING NOTES

1. ALL COLD FORMED METAL FRAMING (CFMF) IS TO CONFORM TO ASTM A653, CQ, GRADE 33 AND HAVE A MINIMUM YIELD POINT OF 33,000 PSI. ALL CFMF TO BE HOT–DIP GALVANIZED FOR A MINIMUM G60 COATING.
2. ALL CFMF, UTILIZED AS THE STRUCTURAL BACKUP FOR THE BUILDING WALLS, WILL CONFORM TO THE FOLLOWING CRITERIA:
   (A) WALL STUDS TO BE SPACED NO FURTHER APART THAN 16 INCHES ON CENTER.
   (B) WALL STUDS TO HAVE THE FOLLOWING SECTION PROPERTIES:
   DEPTH = 6 INCHES
   FLANGE WIDTH = 2 INCHES
   GAGE = 16 MINIMUM
   Ix = 3.340 IN^4 MINIMUM
   Sx = 1.108 IN^3
   (C) WALL STUDS SHALL BE BRACED BY CONTINUOUS MECHANICAL BRIDGING TO FULLY DEVELOP BENDING CAPACITY OF THE STUDS.
   (D) WALL STUDS TO BE ATTACHED TO THE STEEL STRUCTURE AND/OR CONCRETE FLOOR SLABS TO TRANSFER THE SPECIFIED HORIZONTAL LOADS AND TO ACCOMMODATE VERTICAL MOVEMENT OF THE STEEL BEAMS AND SUPPORTED FLOOR SLABS.
   (E) WALL STUD ENDS TO BE ATTACHED TO TRACK COMPONENTS AT THE TOP AND BOTTOM OF THE WALL ASSEMBLY.
3. INSTALLATION OF CFMF IS TO BE IN STRICT ACCORDANCE WITH AISI AND MANUFACTURER'S RECOMMENDATIONS.
4. A MINIMUM OF TWO STUDS ARE TO BE PROVIDED AT THE EDGES OF ALL WALL OPENINGS.
5. CONNECTION OF ALL CFMF MEMBERS IS TO BE MADE PRIOR TO INSTALLATION OF GYPSUM WALLBOARD.
6. TRACK RUNNER MATERIAL THICKNESS SHALL BE AT LEAST EQUAL TO THE TYPICAL WALL STUD MEMBER THICKNESS.
7. NO DEAD LOAD OR LIVE LOAD SHALL BE DIRECTLY IMPOSED ON WALL TRACK OR RUNNER UNLESS SPECIFICALLY DESIGNED.
8. ALL LIGHT GAUGE STEEL MULTIPLE STUD MEMBERS SHALL BE CONNECTED AT 16" O.C. AT EACH FACE OF THE STUD FLANGE.
9. FRAMING MEMBERS SHALL BE INSTALLED ALIGN AND PLUMB.
10. SPLICES IN FRAMING MEMBERS SHALL NOT BE PERMITTED UNLESS SPECIFIED.
11. BOTH FLANGES OF STUDS SHALL BE CONNECTED TO THE TOP AND BOTTOM TRACK.
12. TYPICAL SCREW PATTERN FOR ATTACHMENT OF EXTERIOR GRADE GYPSUM SHEATHING TO LIGHT GAGE FRAMING TO BE 6" O.C. AT THE EDGE AND 8" O.C. WITHIN THE FIELD OF EACH SHEET. SCREWS TO BE GALVANIZED.
13. ALL WELDS AND OTHER CONNECTIONS ARE TO BE TOUCHED UP USING GALVANIZING PAINT (SEE SPECIFICATIONS).
14. FOR ADDITIONAL REQUIREMENTS SEE THE PROJECT'S PLANS AND THE SPECIFICATIONS.

METAL PANEL SYSTEM NOTES

1. METAL PANEL SYSTEM MANUFACTURER SHALL COORDINATE, DESIGN, AND PROVIDE ALL GIRTS, TUBES AND OTHER SUPPORTS REQUIRED TO PROPERLY SUPPORT AND ATTACH THE METAL PANEL SYSTEM TO THE SUPERSTRUCTURE. DESIGN SHALL BE PERFORMED BY A REGISTERED PROFESSIONAL ENGINEER LICENSED IN THE DISTRICT OF COLUMBIA. METAL PANEL SYSTEM MANUFACTURER SHALL MAKE ALLOWANCES FOR ALL BUILDING TOLERANCES BEAM DEFLECTIONS AND TEMPERATURE EXPANSIONS WITH ALL CONNECTIONS AND SHALL COORDINATE WITH OTHER WALL SYSTEM CONTRACTORS AS REQUIRED.
2. METAL PANEL SYSTEM MANUFACTURER IS RESPONSIBLE TO PROVIDE A BOND BREAKER MATERIAL BETWEEN ALL CONNECTIONS OF ALUMINUM AND STRUCTURAL STEEL.
3. THE DESIGN AND DETAILING OF THE METAL PANEL SYSTEM IS THE COMPLETE RESPONSIBILITY OF THE METAL PANEL SYSTEM MANUFACTURER. THE METAL PANEL SYSTEM SHALL BE DESIGNED TO MINIMIZE DEFLECTIONS AS REQUIRED BY THE SPECIFICATIONS. SEE ARCHITECTURAL DRAWINGS FOR LOCATION AND DIMENSIONS.
4. FOR ADDITIONAL REQUIREMENTS, SEE TYPICAL DETAILS AND THE SPECIFICATIONS.

WINDOW WALL SYSTEM NOTES

1. WINDOW WALL SYSTEM MANUFACTURER MUST COORDINATE, DESIGN AND PROVIDE SUPPORTS REQUIRED TO PROPERLY ATTACH WINDOW SYSTEM TO THE SUPERSTRUCTURE. DESIGN MUST BE PERFORMED BY A REGISTERED PROFESSIONAL ENGINEER, LICENSED IN THE DISTRICT OF COLUMBIA.
2. WINDOW WALL SYSTEM MANUFACTURER MUST MAKE ALLOWANCES FOR ALL BUILDING TOLERANCES, BEAM DEFLECTIONS AND TEMPERATURE EXPANSIONS WITH ALL CONNECTIONS AND SHALL COORDINATE WITH OTHER WALL SYSTEM CONTRACTORS AS REQUIRED.
3. WINDOW WALL SYSTEM MANUFACTURER IS RESPONSIBLE TO PROVIDE A BOND BREAKER MATERIAL BETWEEN ALL CONNECTIONS OF ALUMINUM AND STRUCTURAL STEEL.
4. THE DESIGN AND DETAILING OF THE WINDOW WALL SYSTEM IS THE COMPLETE RESPONSIBILITY OF THE WINDOW WALL MANUFACTURER. THE WINDOW WALL SYSTEM MUST BE DESIGNED TO MINIMIZE DEFLECTIONS AS REQUIRED BY THE SPECIFICATIONS. SEE ARCHITECTURAL DRAWINGS FOR LOCATION AND PROFILE.
5. FOR ADDITIONAL REQUIREMENTS SEE ARCHITECTURAL DRAWINGS AND SPECIFICATIONS.

SPECIAL INSPECTION NOTES

1. SPECIAL INSPECTIONS ARE REQUIRED IN ACCORDANCE WITH IBC SECTION 1704
   A. INSPECTION OF EARTHWORK
   B. INSPECTION OF CAST IN PLACE CONCRETE / REINFORCEMENT
   C. INSPECTION OF STRUCTURAL STEEL
   D. INSPECTION OF SPRAY FIREPROOFING
2. SEE SPECIFICATION SECTION 014100 FOR ADDITIONAL INSPECTION REQUIREMENTS.

STRUCTURAL DRAWING INDEX

Drawing	Title	75% CD SUBMISSION MARCH 16, 2011	95% CD SUBMISSION FEBRUARY 17, 2012	ISSUE 1 READY TO ADVERTISE JUNE 26, 2012	ISSUE 1 ISSUE FOR CONSTRUCTION APRIL 30, 2013	
SS001	STRUCTURAL NOTES AND INDEX SHEET	X	X	X	X	
SS201	FOUNDATION PLAN	X	X	X	X	
SS202	ROOF AND PENTHOUSE FRAMING PLANS	X	X	X	X	
SS203	ROOF AND PENTHOUSE REINFORCEMENT PLANS AND DETAILS	X	X	X	X	
SS301	SECTIONS	X	X	X	X	
SS401	COLUMN SCHEDULE AND DETAILS	X	X	X	X	
SS501	TYPICAL DETAILS	X	X	X	X	
SS502	TYPICAL DETAILS	X	X	X	X	

X "ISSUED FOR BID"
A "ISSUED FOR REFERENCE ONLY"

DESIGN LIVE LOADS IBC 2006

FLOOR DESIGN LIVE LOADS

PENTHOUSE FLOOR 150 PSF

ROOF DESIGN LIVE LOADS

TYPICAL ROOF 30 PSF
MAXIMUM DRIFT LOAD (FOR AREAS OF SNOW BUILD UP) 87 PSF

SNOW LOADS

GROUND SNOW LOAD,	Pg=25 PSF
FLAT ROOF SNOW LOAD,	Pf=20 PSF
SNOW EXPOSURE FACTOR,	Ce=1.0
SNOW LOAD IMPORTANCE FACTOR,	I=1.0
THERMAL FACTOR,	Ct=1.0

LATERAL LOADS - WIND

WIND LOAD DESIGN PARAMETERS:

BASIC WIND SPEED,	V=90 MPH
WIND LOAD IMPORTANCE FACTOR	I=1.0
WIND EXPOSURE	B
INTERNAL PRESSURE COEFFICIENT,	GCpi=±0.18

WIND LOAD ON STRUCTURAL FRAME:

HEIGHT	NORTH/SOUTH DIRECTION	EAST/WEST DIRECTION
0 FEET TO 15 FEET	12 PSF	11 PSF
15 FEET TO 20 FEET	12 PSF	12 PSF

WIND LOADS ON COMPONENTS AND CLADDING:

COMPONENT LOCATION **	
TYPICAL WALL	16 PSF
WALL CORNERS	20 PSF
TYPICAL ROOF	15 PSF
ROOF PERIMETER	25 PSF
ROOF CORNER	37 PSF
PARAPET	47 PSF

** NOTE: COMPONENT WIND LOADS ARE BASED ON A TRIBUTARY AREA OF 10 SQ. FT. VALUES MAY BE ADJUSTED PROVIDED WIND LOAD CALCULATIONS ARE SUBMITTED FOR REVIEW.

LATERAL LOADS - SEISMIC

SEISMIC LOAD INFORMATION FOR STRUCTURAL FRAME:

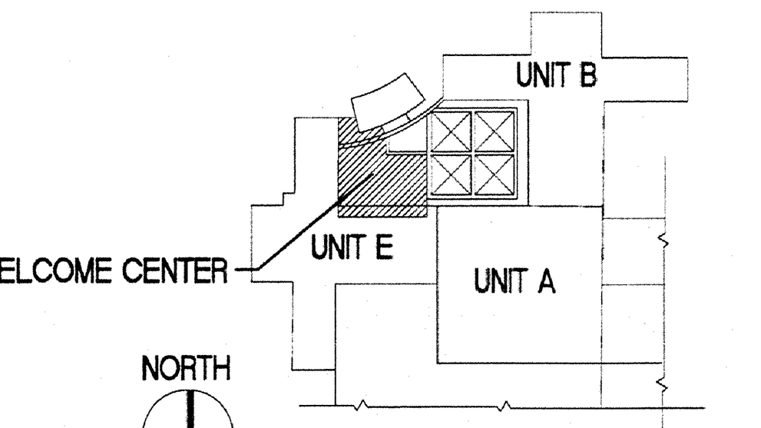
SEISMIC OCCUPANCY CATEGORY	II
SEISMIC IMPORTANCE FACTOR,	I=1.00
SHORT PERIOD MAPPED SPECTRAL RESPONSE ACCELERATION	$S_s=0.153$
1–SECOND MAPPED SPECTRAL RESPONSE ACCELERATION	$S_1=0.05$
LONG PERIOD TRANSITION PERIOD	$T_L=8$
SITE CLASS	D
SHORT PERIOD SPECTRAL RESPONSE COEFFICIENT	$S_{DS}=0.163$
1–SECOND PERIOD SPECTRAL RESPONSE COEFFICIENT	$S_{D1}=0.08$
SEISMIC DESIGN CATEGORY,	B
ANALYSIS PROCEDURE	EQUIV. LAT. FORCE
BASIC SEISMIC–FORCE–RESISTING SYSTEM ORDINARY STEEL	MOMENT FRAME
RESPONSE MODIFICATION FACTOR,	R=3 *
SEISMIC RESPONSE COEFFICIENT	Cs=0.054
DESIGN BASE SHEAR	27 KIPS

* LATERAL SYSTEM NOT REQUIRED TO BE SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE IN ACCORDANCE WITH AISC 341 OR AISI LATERAL.

BLAST LOADS

LOADS DEFINED IN VA PHYSICAL SECURITY DESIGN MANUAL FOR LIFE–SAFETY PROTECTED STRUCTURES (2007):

TYPICAL FACADE, ROOF, AND GLAZING LOAD: GP1



KEY PLAN

Revisions	Date
ISSUE 1 - ISSUE FOR CONSTRUCTION	04.30.2013
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Drawing Title: STRUCTURAL NOTES AND INDEX SHEET

Approved: Project Director

Project Title: OIF / OEF WELCOME CENTER DEPARTMENT OF VETERANS AFFAIRS VAMC

Location: Veterans Affairs Medical Center, 50 Irving Street NW Washington DC

Date: 4-30-2013 | Checked: JRM | Drawn: BEC

Project Number: 688-334 OIF/OEF

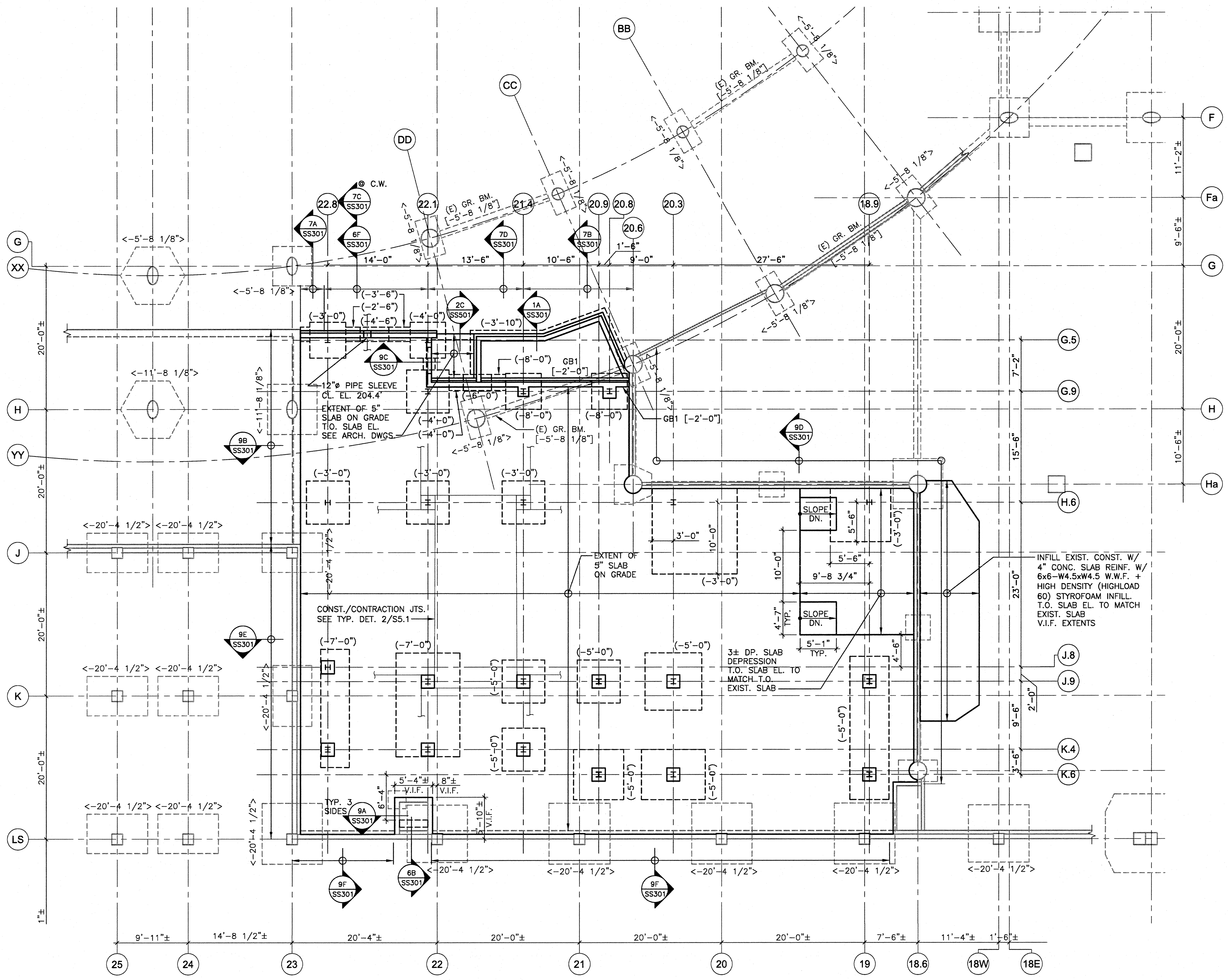
Building Number:

Drawing Number: SS001

Office of Construction and Facilities Management

Department of Veterans Affairs

VA FORM 08-6231



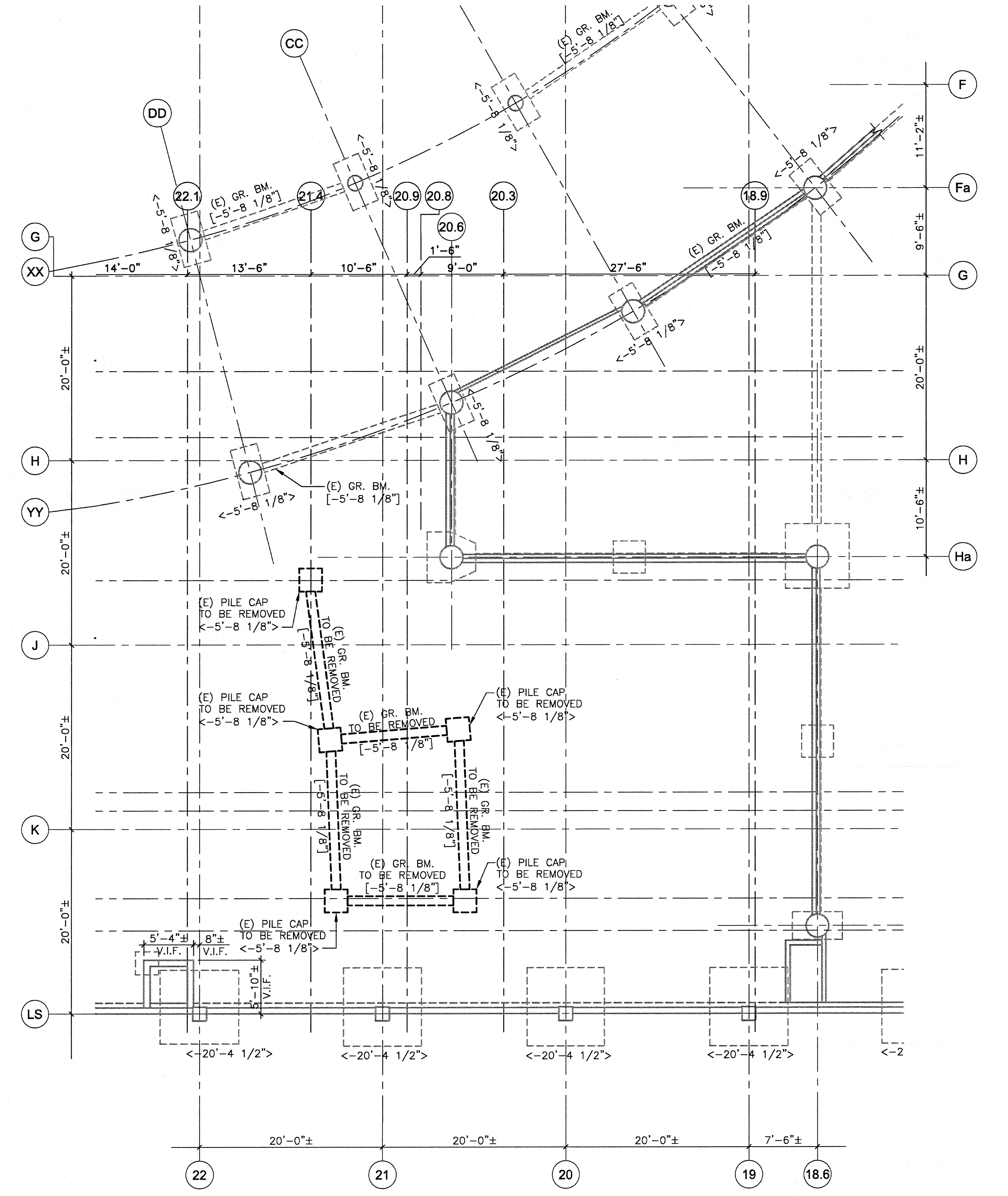
3E FOUNDATION PLAN

1/8" = 1'–0"

SS201

(UNLESS NOTED OTHERWISE)

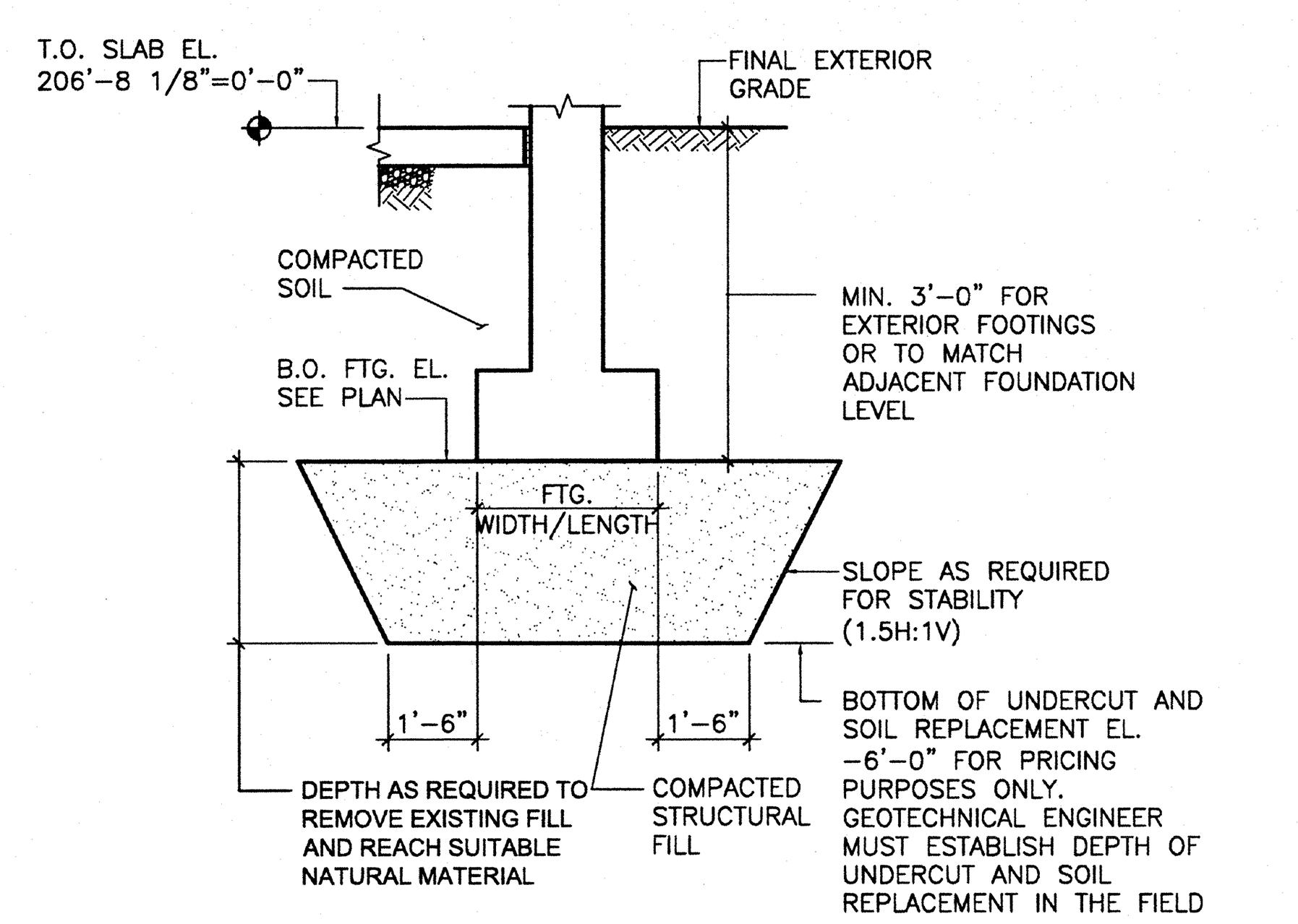
1. TOP OF NEW FLOOR SLAB ELEVATION 0'–0". ACTUAL ELEVATION 206'–8 1/8". NEW SLAB TO MATCH EXISTING TOP OF SLAB ELEVATION.
2. WHERE INDICATED IN PLAN, PROVIDE A 5 INCH CONCRETE SLAB ON GRADE REINFORCED WITH 6x6–W5.5xW5.5 W.W.F.
3. BOTTOM OF NEW FOOTING ELEVATION INDICATED THUS (...) IN PLAN. BOTTOM OF EXISTING FOOTING ELEVATION INDICATED THUS (±...) IN PLAN.
4. TOP OF NEW GRADE BEAM ELEVATION INDICATED THUS [...] IN PLAN. TOP OF EXISTING GRADE BEAM ELEVATION INDICATED THUS [±...] IN PLAN.
5. TOP OF EXISTING PILE CAP ELEVATION INDICATED THUS <±...> IN PLAN.
6. CONSTRUCTION MANAGER/GENERAL CONTRACTOR TO COORDINATE SIZE AND LOCATION OF ALL WALL OPENINGS AND/OR SLEEVES WITH ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS AND TYPICAL DETAILS.
7. SEE DRAWING SS001 FOR ADDITIONAL NOTES AND DRAWINGS SS501 AND SS502 FOR TYPICAL DETAILS.



2 EXISTING FOUNDATION DEMOLITION PLAN

1/8" = 1'–0"

SS201



NOTE:
1. SEE EARTHMOVING SPECIFICATION SECTION AND GEOTECHNICAL REPORT FOR ADDITIONAL INFO.
2. EXTENT OF FOUNDATION OVER–EXCAVATION MAY BE MODIFIED IN FIELD BY GEOTECHNICAL ENGINEER FOR STABILITY CONSIDERATIONS.

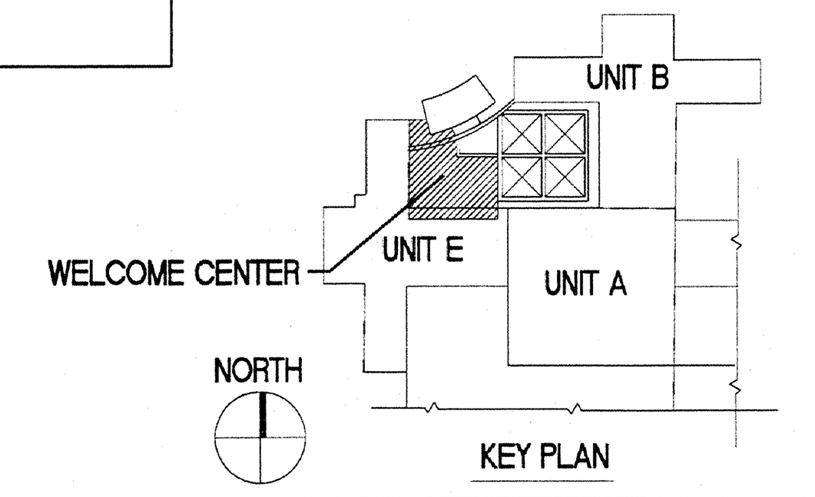
7F TYPICAL DETAIL – FOUNDATION UNDERCUTTING

NO SCALE

SS201

PRICING NOTES:

1. PROVIDE ALLOWANCE FOR PARTIAL DEMOLITION OF ABANDONED FOUNDATIONS AT LOCATIONS OF NEW FOUNDATIONS.
2. FOR ENTIRE FOOTPRINT OF NEW ADDITION, A TWO FOOT UNDERCUT AND BACKFILL WITH NEW COMPACTED FILL WILL BE REQUIRED.
3. FOUNDATIONS LOCATED ADJACENT TO EXISTING BASEMENT WALLS ARE TO BE SET LOWER TO BE ON NATURAL SOILS AND TO LIMIT SURCHARGE ON EXISTING BASEMENT WALL. SEE PLAN FOR APPROXIMATE BOTTOM OF FOOTING ELEVATIONS.
4. BALANCE OF FOUNDATIONS TO RECEIVE AN UNDERCUT DOWN TO NATURAL SOILS WITH AN EXCAVATION SLOPE OF 1.5H:1V OR LESS AND NEW COMPACTED FILL PLACED BACK TO REACH BOTTOM OF FOOTING ELEVATIONS SHOWN ON PLAN. SEE DETAIL A/SS201 FOR ADDITIONAL INFORMATION. FOR PRICING PURPOSES, ASSUME AN UNDERCUT OF FOUR FEET ADDITIONAL TO THE TWO FEET IN NOTE 2.



KEY PLAN

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CONSULTANTS:

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Drawing Title: FOUNDATION PLAN

Approved: Project Director

Project Title: OIF / OEF WELCOME CENTER DEPARTMENT OF VETERANS AFFAIRS VAMC

Location: Veterans Affairs Medical Center, 50 Irving Street NW Washington DC

Date: 4-30-2013 | Checked: JRM | Drawn: BEC

Project Number: 688-334 OIF/OEF

Building Number:

Drawing Number: SS201

Office of Construction and Facilities Management

Department of Veterans Affairs

VA FORM 08–6231

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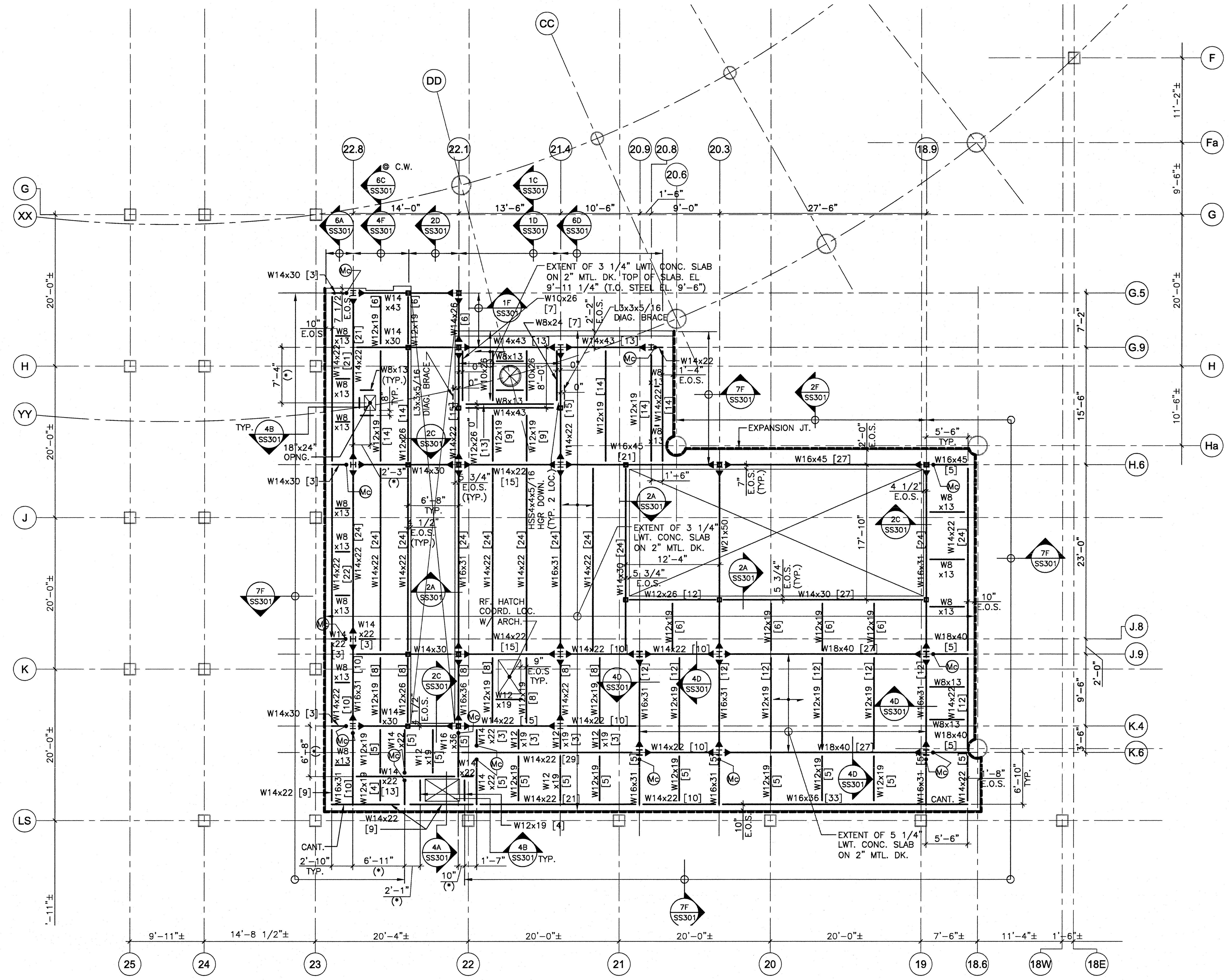
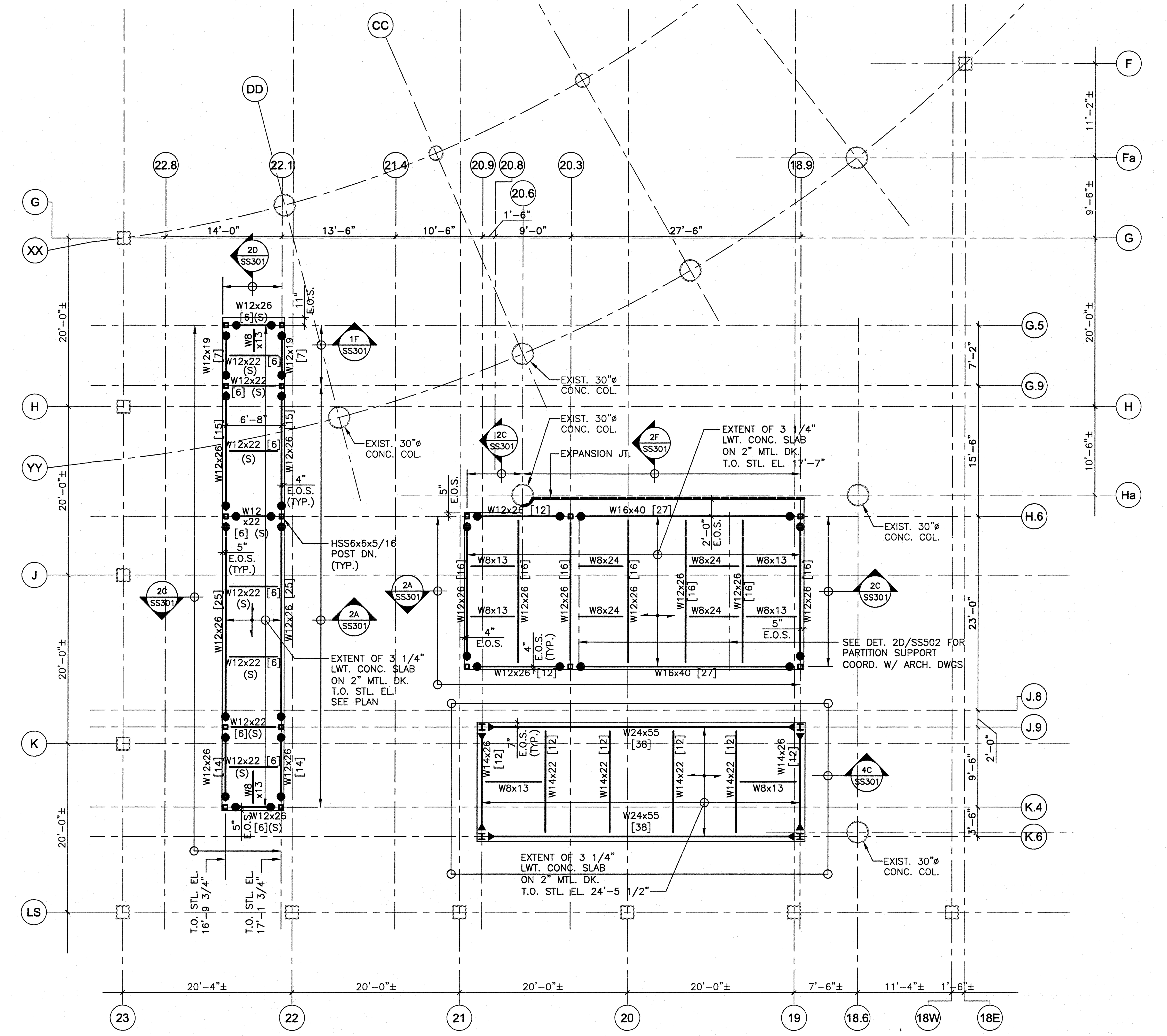


(2E) ROOF FRAMING PLAN

1/8" = 1'-0" SS202

1. TOP OF STEEL ELEVATION 12'-4". ELEVATIONS INDICATED THUS (±) ARE RELATIVE TO ELEVATION 12'-4".



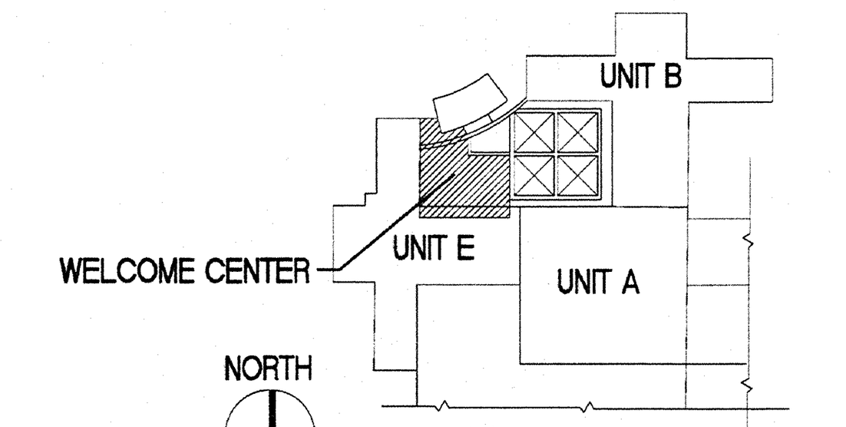
(7D) PENTHOUSE FLOOR AND MONITOR ROOFS FRAMING PLANS

1/8" = 1'-0" SS202

1. TOP OF STEEL ELEVATION SEE PLAN.

GENERAL NOTES
(UNLESS NOTED OTHERWISE)

1. TOP OF STEEL ELEVATION SEE PLAN.
2. PROVIDE 3-1/4 INCH LIGHTWEIGHT CONCRETE SLAB ON 2 INCH - 20 GAUGE GALVANIZED COMPOSITE METAL DECK. SEE DRAWING SS203 FOR SLAB REINFORCEMENT. FOR SPAN OF DECK SEE PLAN.
3. PROVIDE 5-1/4" INCH LIGHTWEIGHT CONCRETE SLAB ON 2 INCH - 20 GAUGE GALVANIZED COMPOSITE METAL DECK. SEE DRAWING SS203 FOR SLAB REINFORCEMENT. FOR SPAN OF DECK SEE PLAN.
4. TEXT INDICATED THUS [...] IN PLAN INDICATES THE NUMBER OF EQUALLY SPACED 3/4 INCH DIAMETER x 4 INCH LONG HEADED SHEAR STUDS WELDED TO THE TOP FLANGE CENTERLINE OF BEAMS.
5. PROVIDE MOMENT CONNECTIONS AT ALL BEAM/COLUMN & BEAM/BEAM JOINTS INDICATED THUS ▶ IN PLAN. SEE TYPICAL DETAIL 1F/SS401.
6. PROVIDE MOMENT CONNECTIONS AT ALL BEAM/HSS POST JOINTS INDICATED THUS ● IN PLAN. SEE TYPICAL DETAIL 2F/SS402.
7. GENERAL CONTRACTOR TO COORDINATION DIMENSIONS MARKED THUS (*) IN PLAN WITH MECHANICAL DRAWINGS AND EQUIPMENT PURCHASED.
8. SEE DRAWING SS001 FOR ADDITIONAL NOTES AND DRAWINGS SS501 AND SS502 FOR TYPICAL DETAILS.



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Drawing Title: ROOF AND PENTHOUSE FRAMING PLANS

Approved: Project Director

Project Title: OIF / OEF WELCOME CENTER DEPARTMENT OF VETERANS AFFAIRS VAMC

Project Number: 688-334 OIF/OEF

Building Number

Location: Veterans Affairs Medical Center, 50 Irving Street NW Washington DC

Drawing Number: SS202

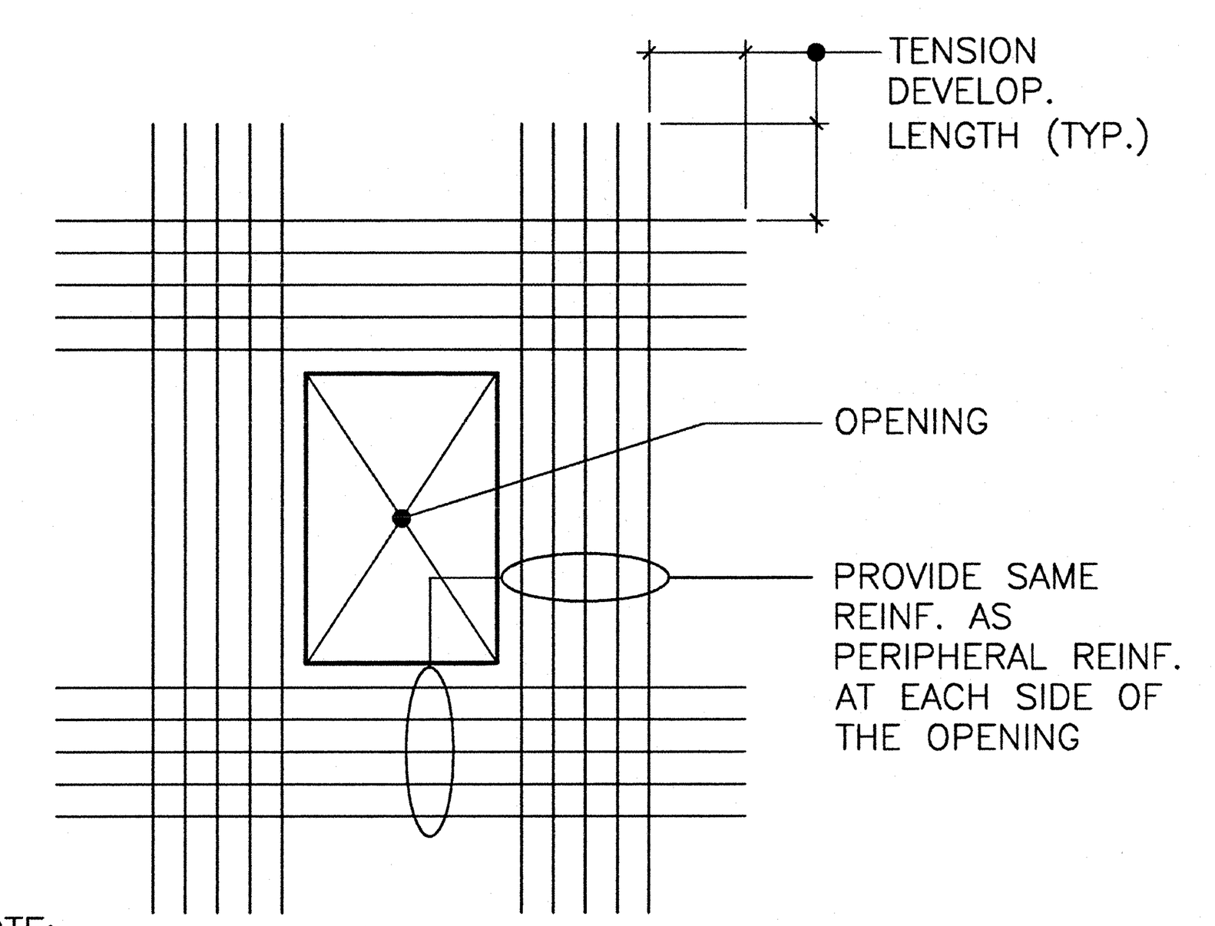
Date: 4-30-2013 | Checked: JRM | Drawn: BEC

Office of Construction and Facilities Management

Department of Veterans Affairs

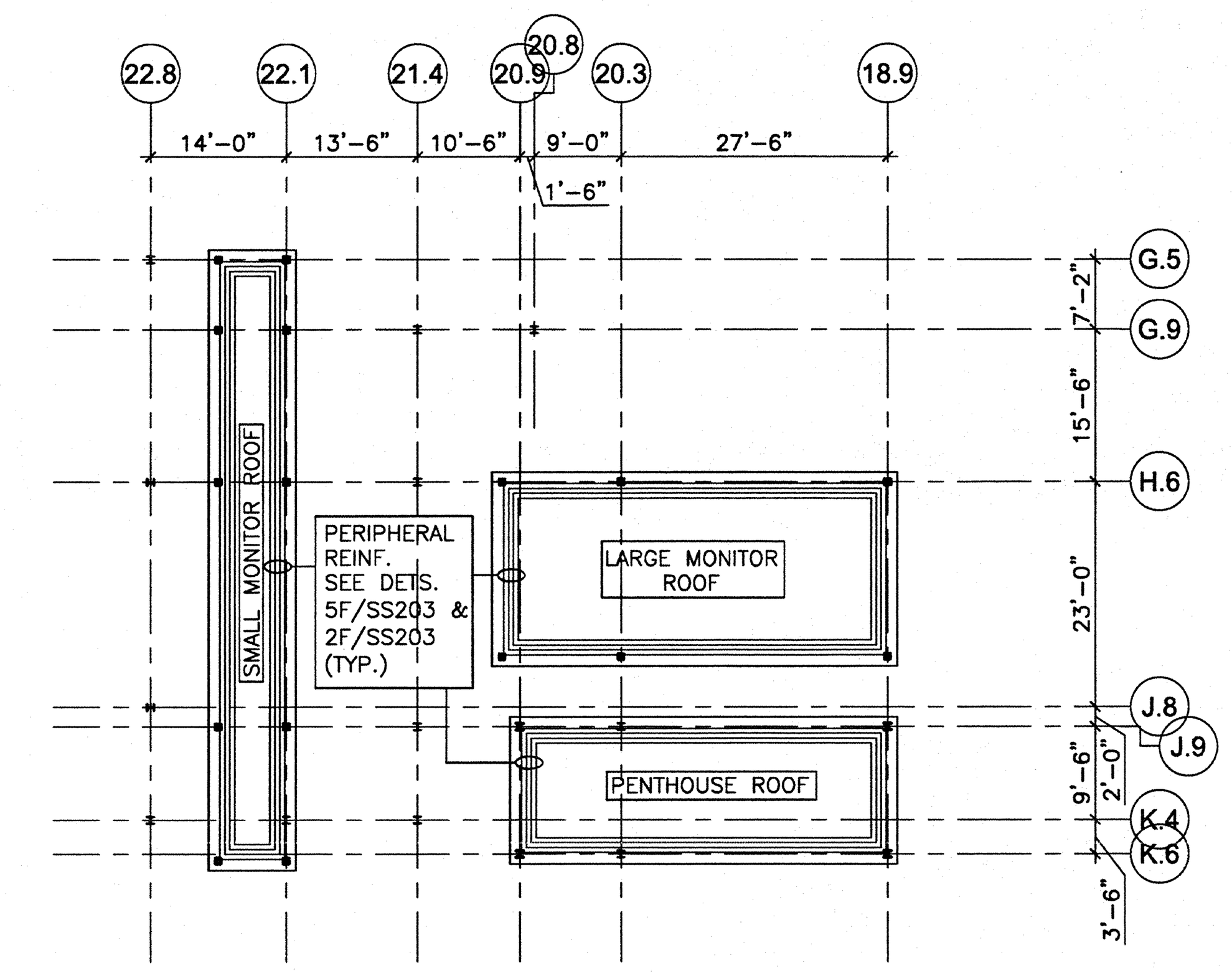
VA FORM 08-6231

SLAB REINFORCEMENT		
LEVEL	INTERNAL REINF.	PERIPHERAL REINF.
MAIN ROOF	#4@12" E.W.	(6)–#4
PENTHOUSE FLOOR	#4@9" E.W.	(6)–#6
SMALL MONITOR ROOF	#4@12" E.W.	(4)–#4
LARGE MONITOR ROOF	#4@12" E.W.	(5)–#4
PENTHOUSE ROOF	#4@12" E.W.	(6)–#4
LOW ROOF	#4@12" E.W.	(4)–#4

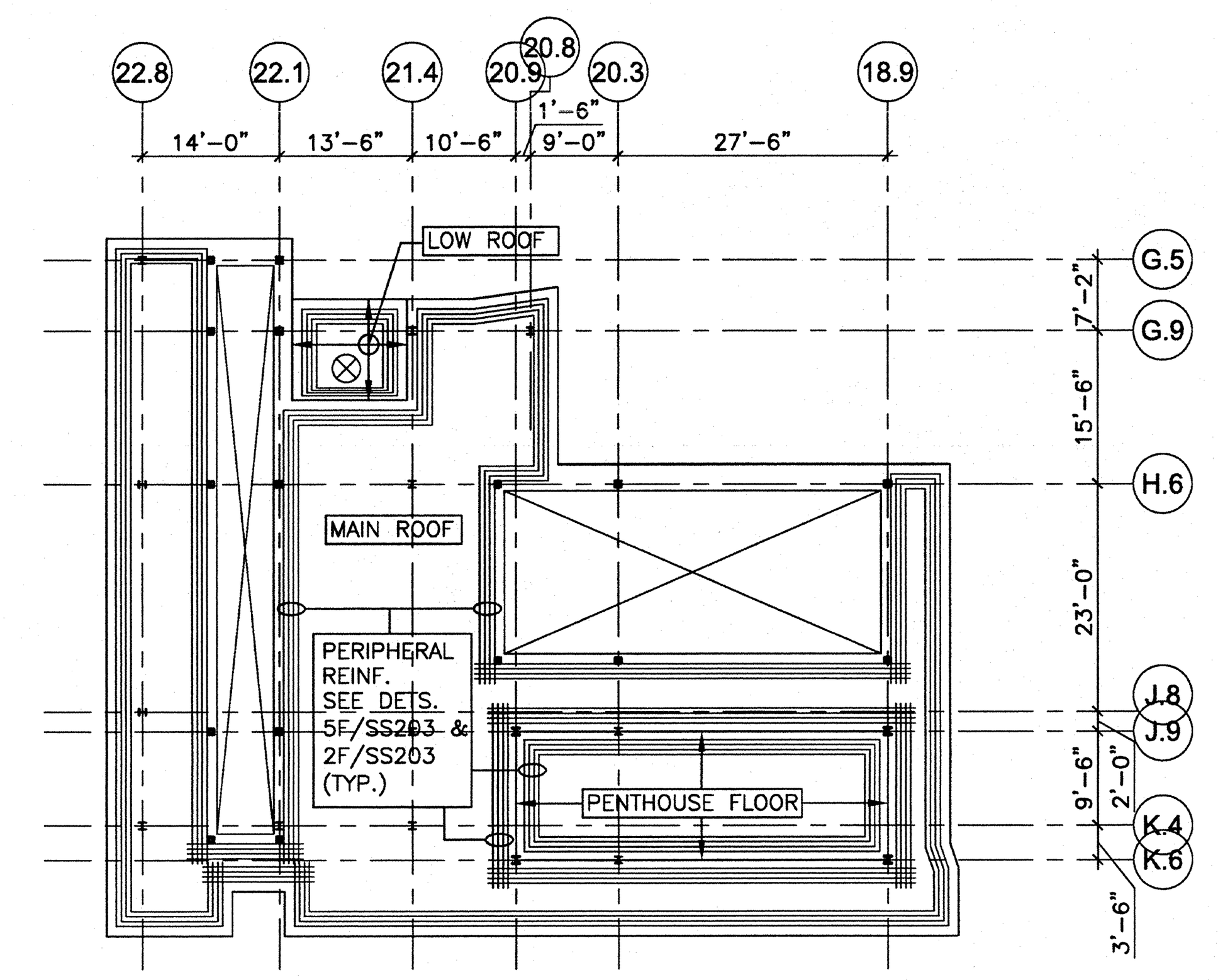


NOTE:
NOT ALL OPENINGS ARE SHOWN ON THE STRUCTURAL DRAWINGS. THE CONTRACTOR SHALL COORDINATE ALL OPENINGS SHOWN ON THE ARCHITECTURAL, STRUCTURAL, MECHANICAL, PLUMBING, AND ELECTRICAL DRAWINGS.

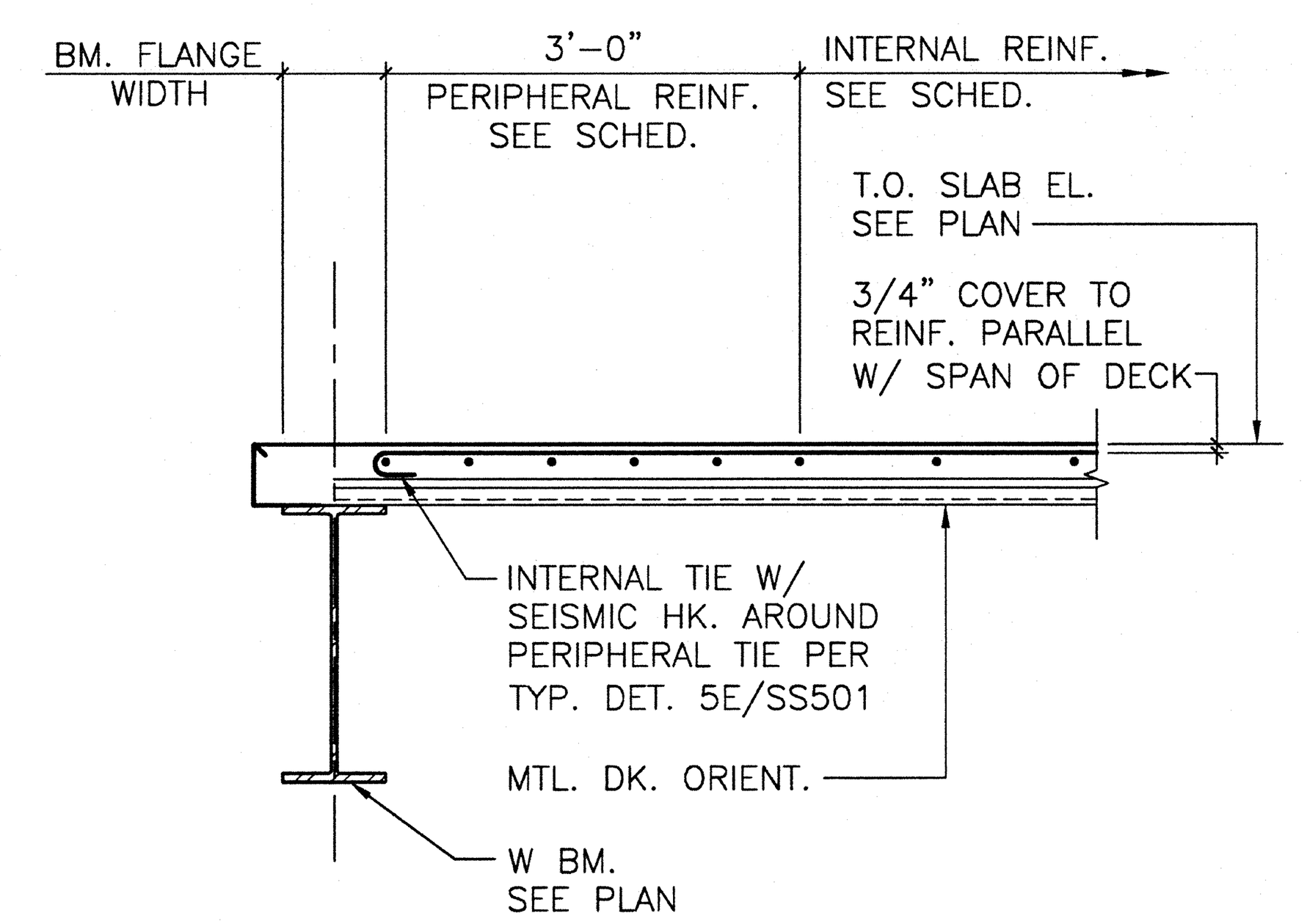
5D TYPICAL PLAN DETAIL – REINFORCEMENT AT SLAB OPENINGS
NO SCALE SS203



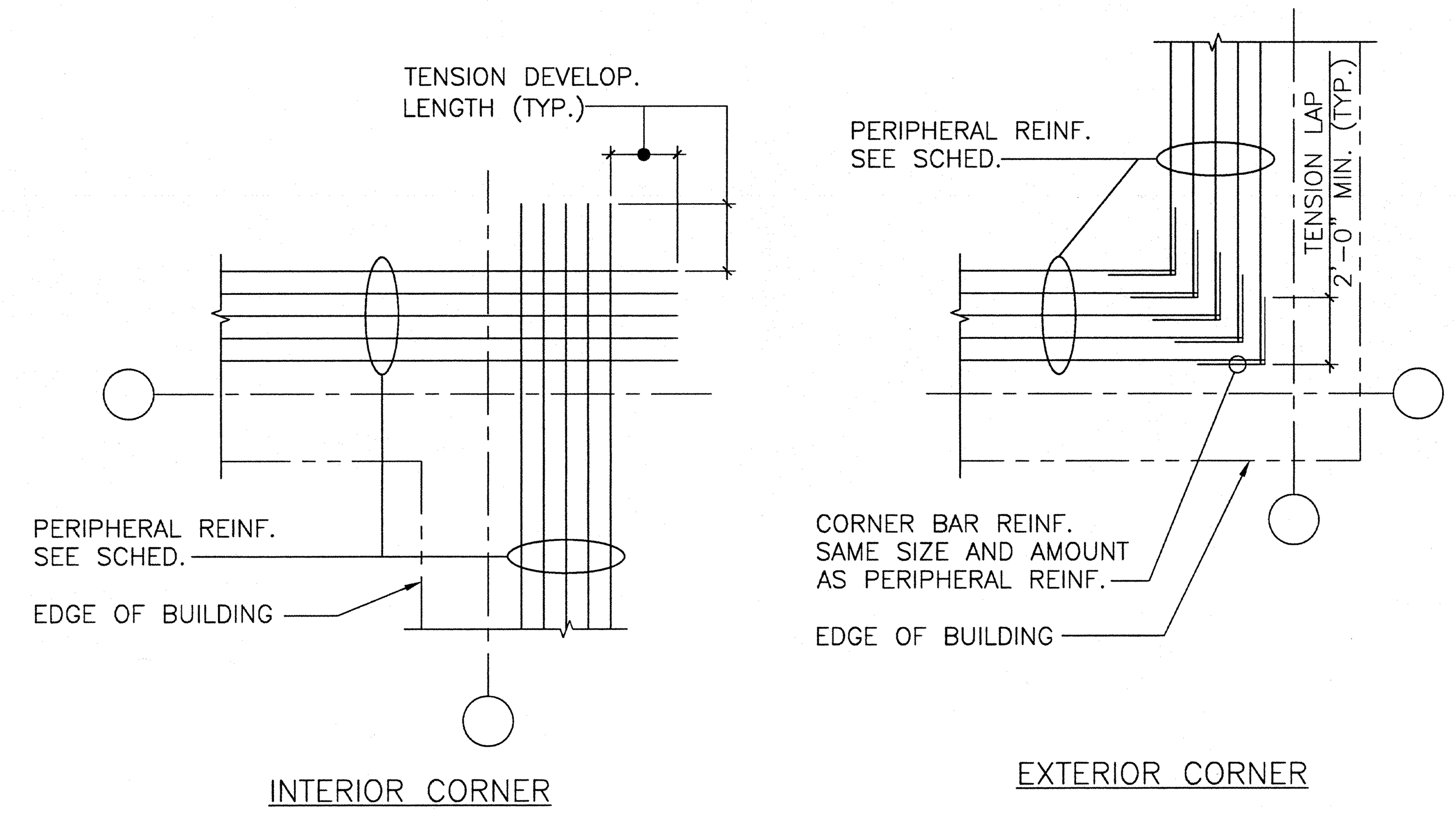
7B PENTHOUSE AND MONITOR ROOFS REINFORCMENT PLANS
1/16" = 1'–0" SS203



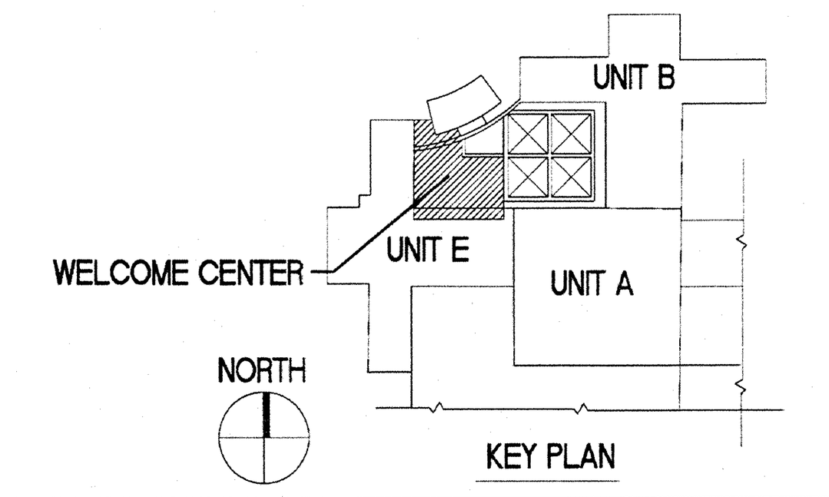
7F ROOF REINFORCEMENT PLAN
1/16" = 1'–0" SS203



2F TYPICAL SLAB SECTION
NO SCALE SS203



5F TYPICAL PLAN DETAIL – CORNER CONDITION
NO SCALE SS203



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CONSULTANTS:

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Drawing Title: ROOF AND PENTHOUSE REINFORCEMENT PLANS AND DETAILS

Approved: Project Director

Project Title: OIF / OEF WELCOME CENTER DEPARTMENT OF VETERANS AFFAIRS VAMC

Location: Veterans Affairs Medical Center, 50 Irving Street NW Washington DC

Date: 4-30-2013 | Checked: JRM | Drawn: BEC

Project Number: 688-334 OIF/OEF

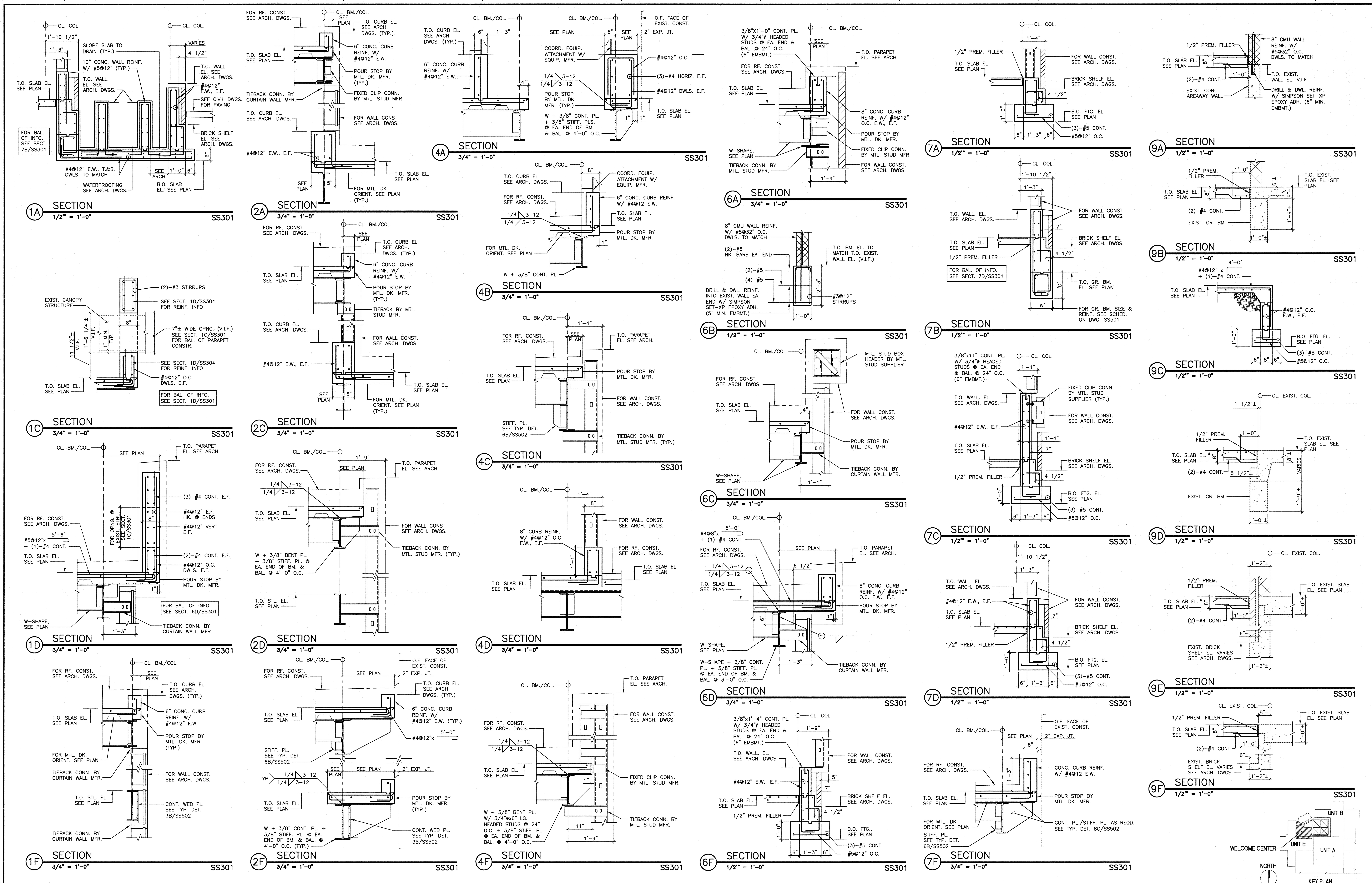
Building Number:

Drawing Number: SS203

Office of Construction and Facilities Management

Department of Veterans Affairs

VA FORM 08–6231



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Drawing Title: SECTIONS

Approved: Project Director

Project Title: OIF / OEF WELCOME CENTER DEPARTMENT OF VETERANS AFFAIRS VAMC

Location: Veterans Affairs Medical Center, 50 Irving Street NW Washington DC

Date: 4-30-2013 | Checked: JRM | Drawn: BEC

Project Number: 688-334 OIF/OEF

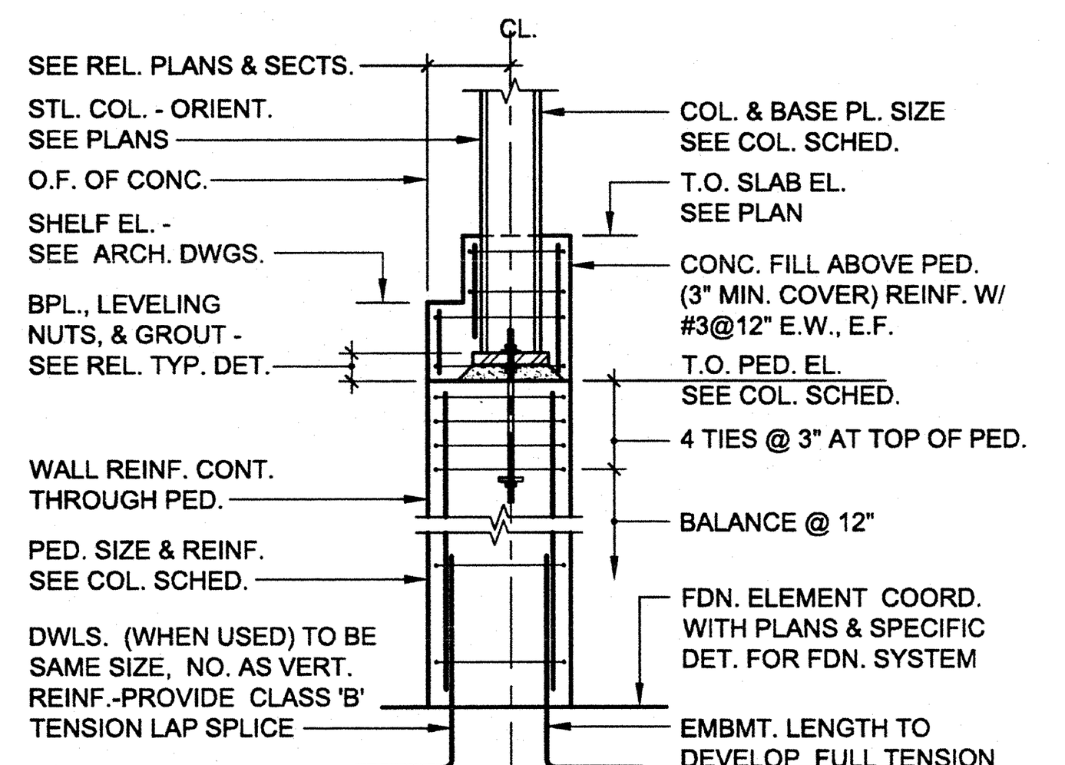
Building Number

Drawing Number: SS301

Office of Construction and Facilities Management

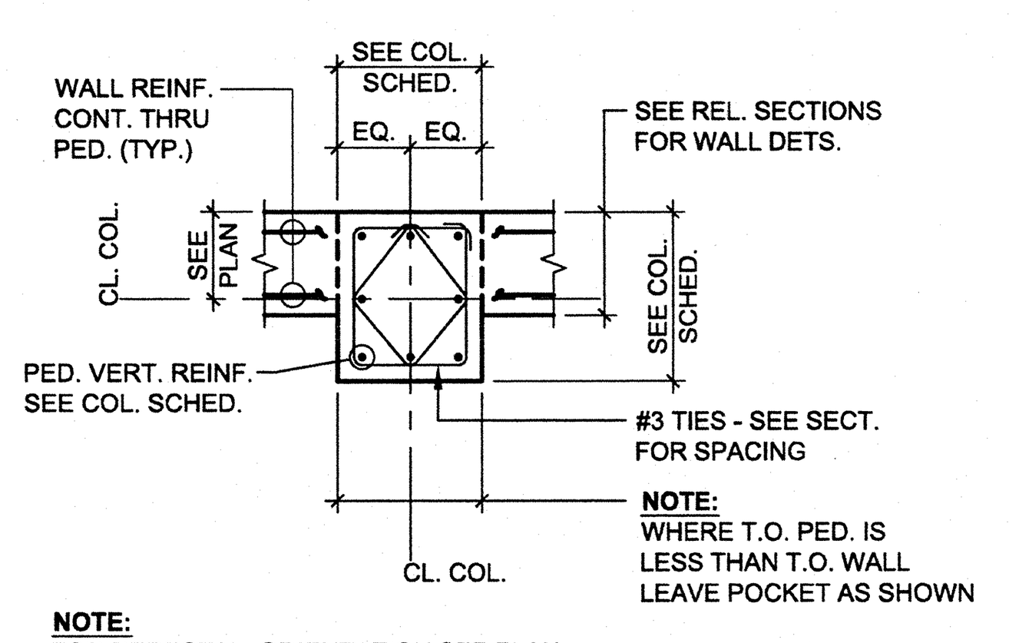
Department of Veterans Affairs

VA FORM 08-6231



NOTES:
1. COORD. W/ TYP. DET.-COL. PED. PLAN VIEWS.
2. COORD. WITH REL. SECTS. & DETS. FOR ADDTL. INFO. NOT SHOWN FOR CLARITY.
3. FILL ABOVE PED. AROUND COL. TO CONFORM WITH LINES OF ADJACENT CONC. WALLS, ARCH. WALL SHELF EL., ETC. CONC. FILL TO BE NORMAL WEIGHT 4000 PSI CONC. WITH A 3" MIN. COVER ALL AROUND.

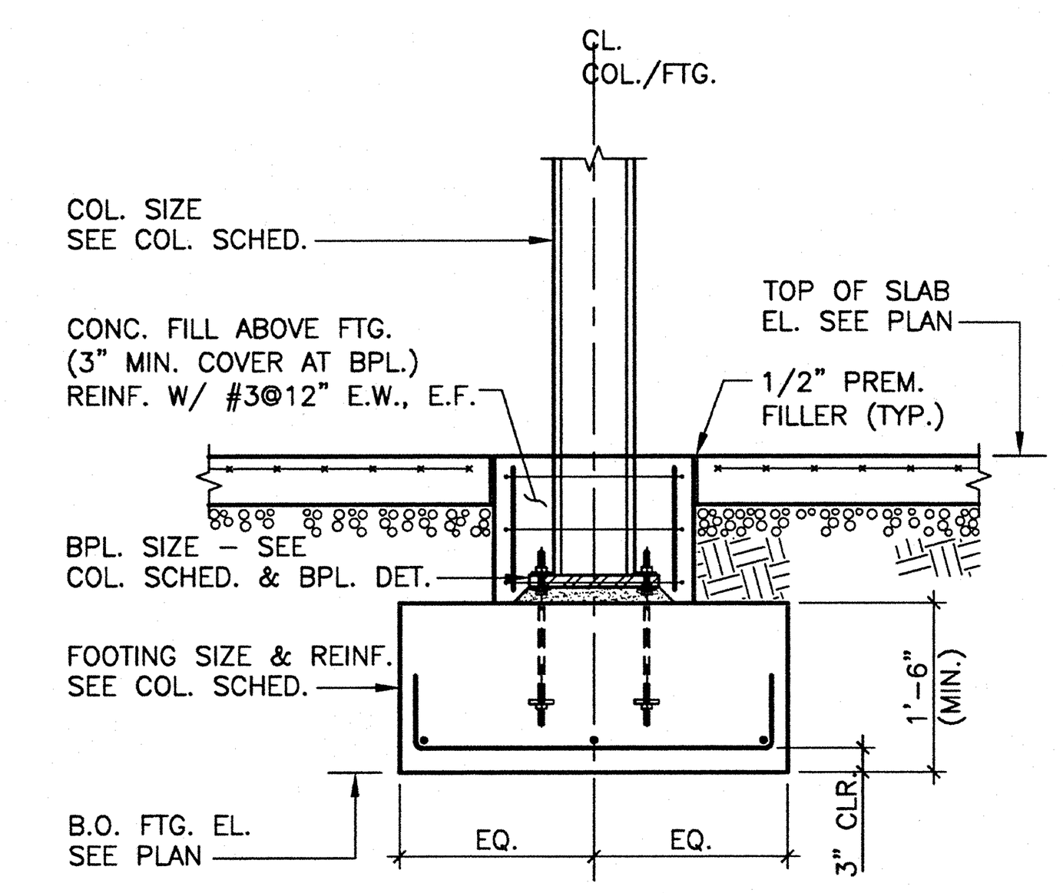
2A TYPICAL DETAIL - PEDESTAL REINFORCING
NO SCALE SS401



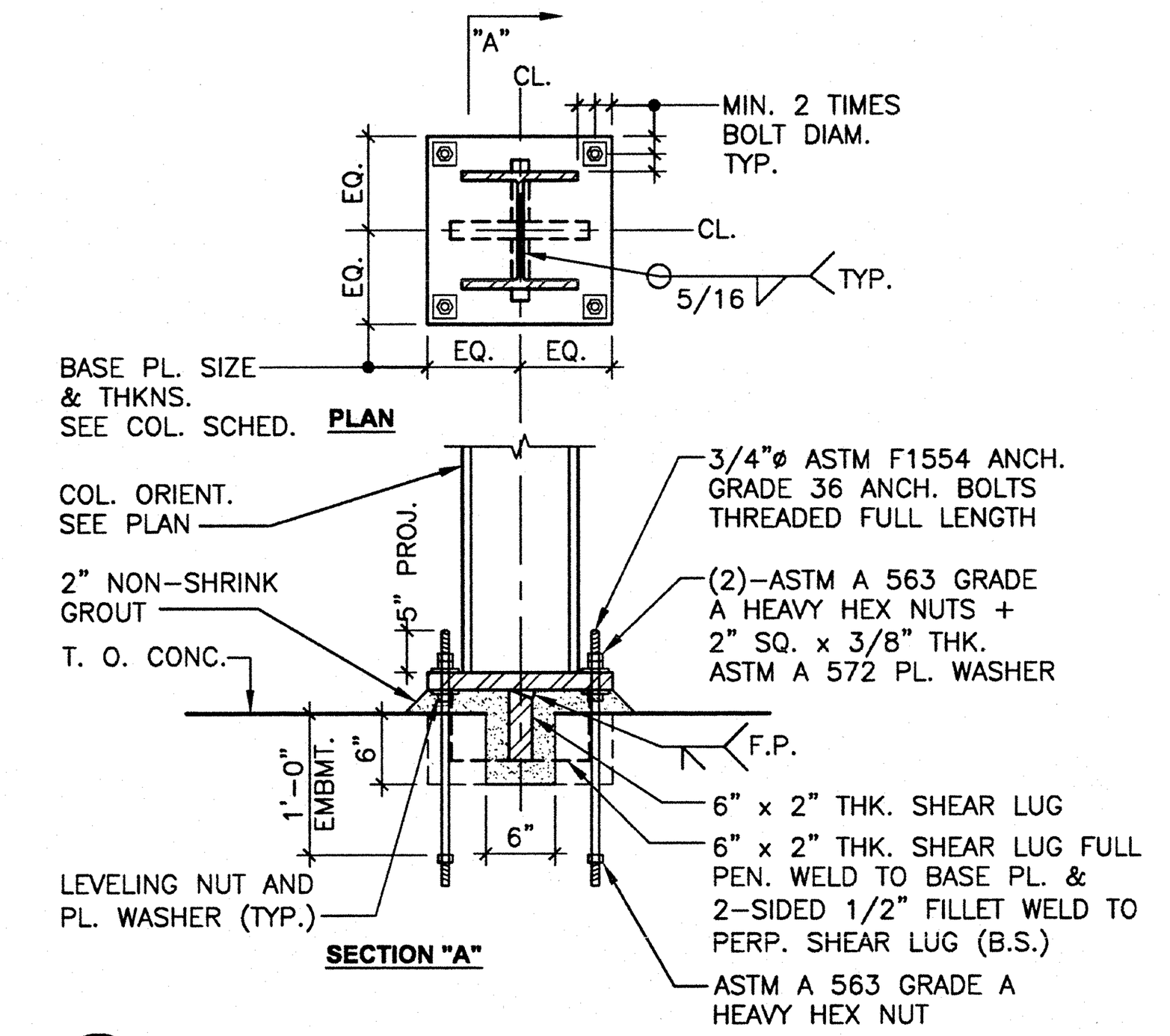
NOTE: WHERE T.O. PED. IS LESS THAN T.O. WALL LEAVE POCKET AS SHOWN

NOTE: FOR PEDESTAL ORIENTATION SEE PLAN

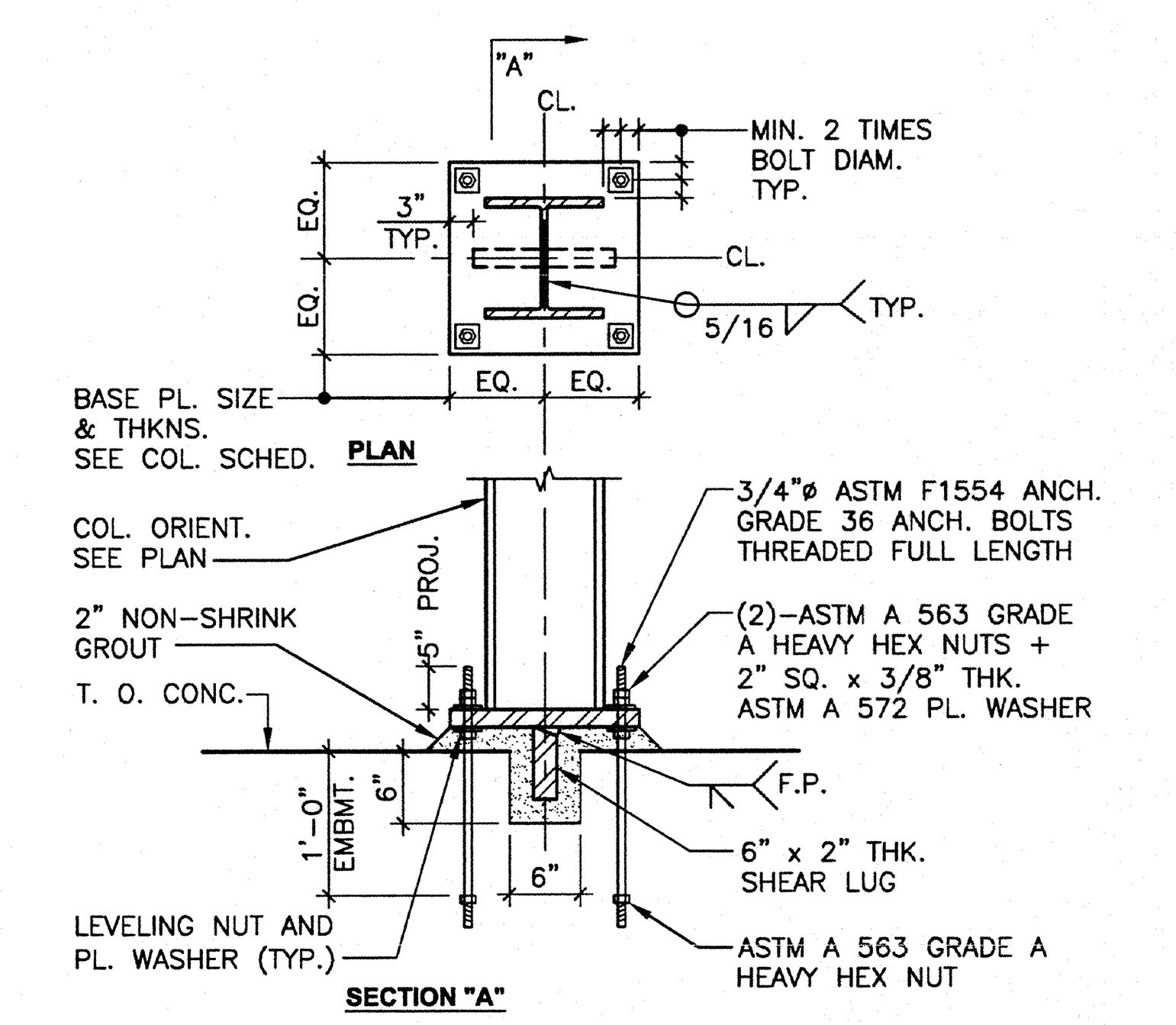
4A TIE/PEDESTAL DETAIL 'B'
NO SCALE SS401



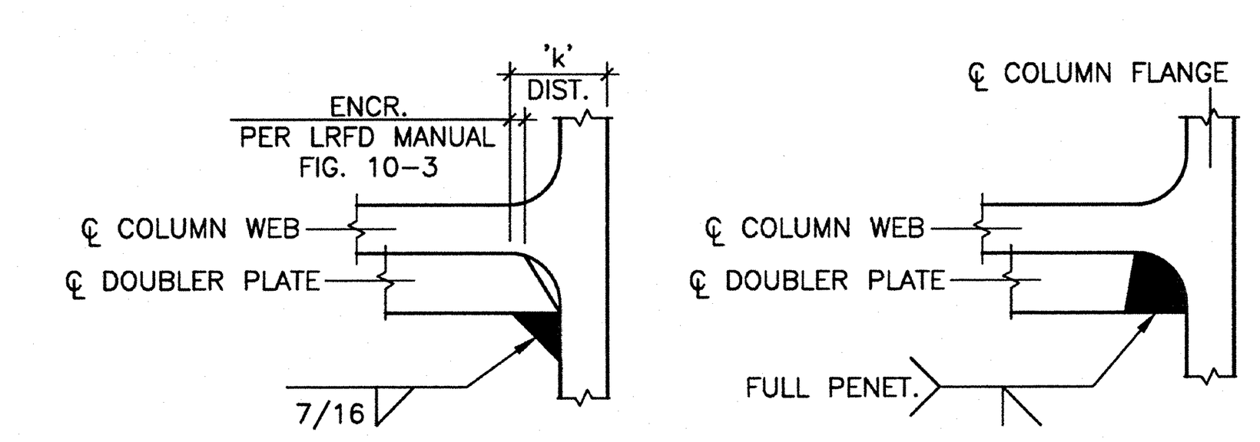
6A TYPICAL DETAIL - COLUMN FOOTING WITHOUT PEDESTAL
NO SCALE SS401

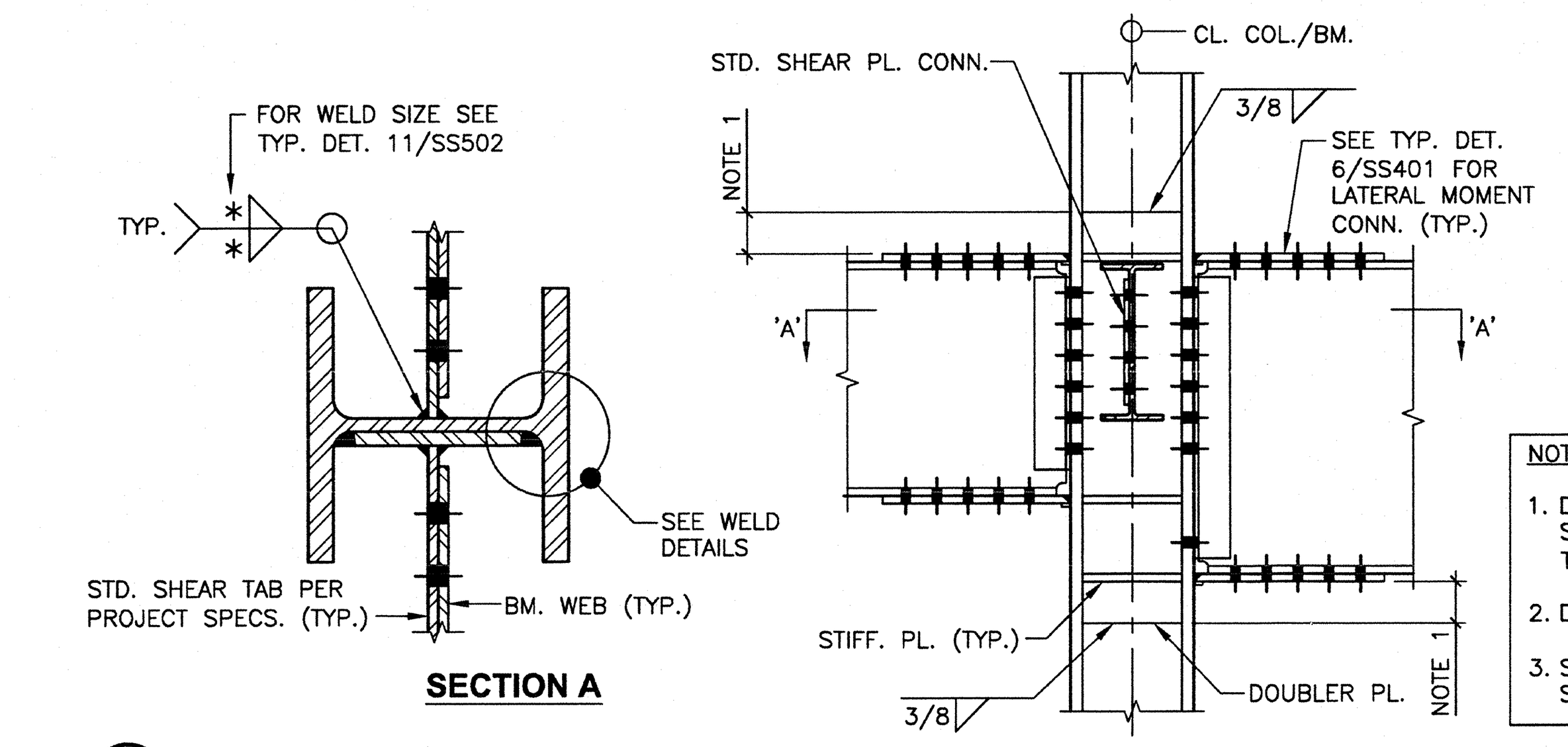


7A TYPICAL DETAIL - COLUMN BASE PLATE TYPE 'D'
NO SCALE SS401



9A TYPICAL DETAIL - COLUMN BASE PLATE TYPE 'B'
NO SCALE SS401

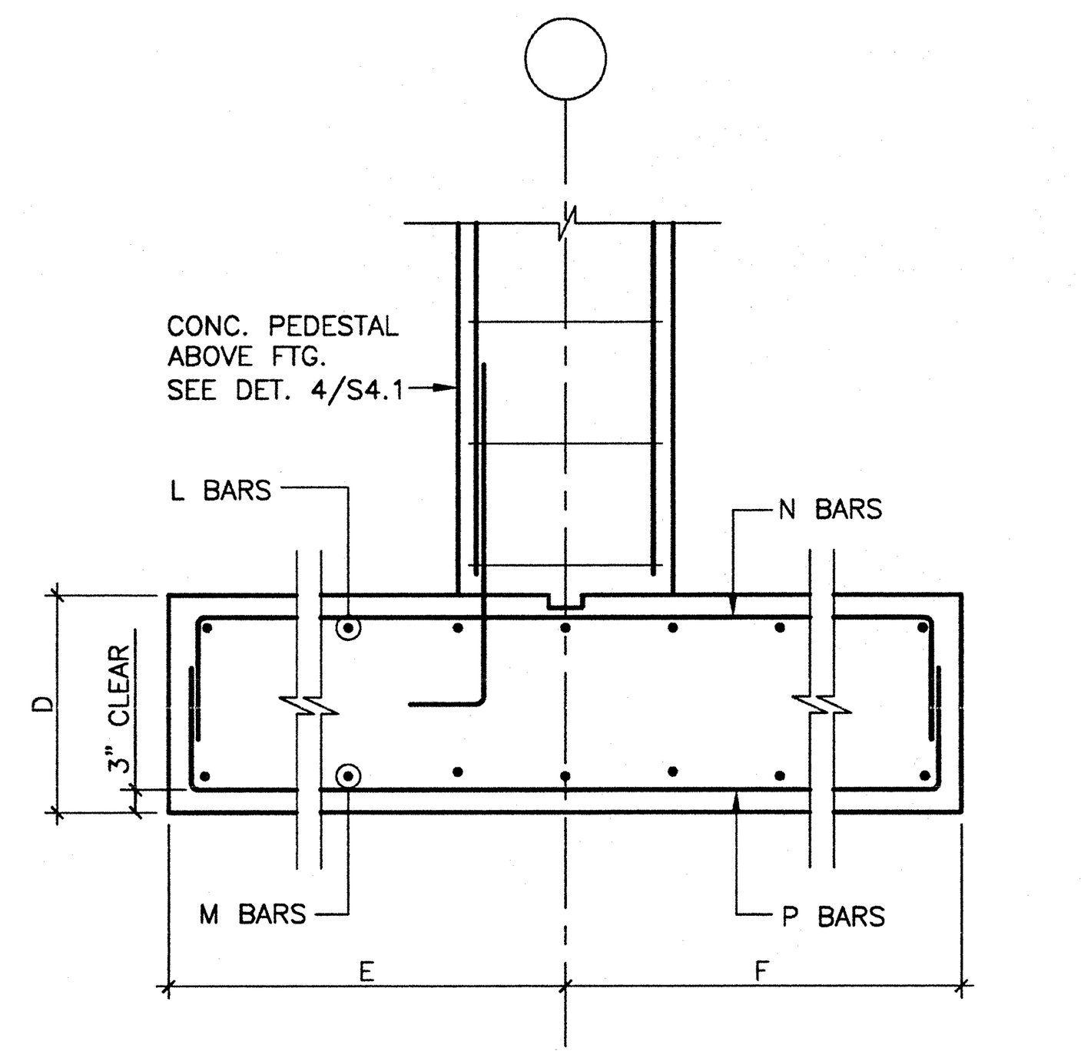




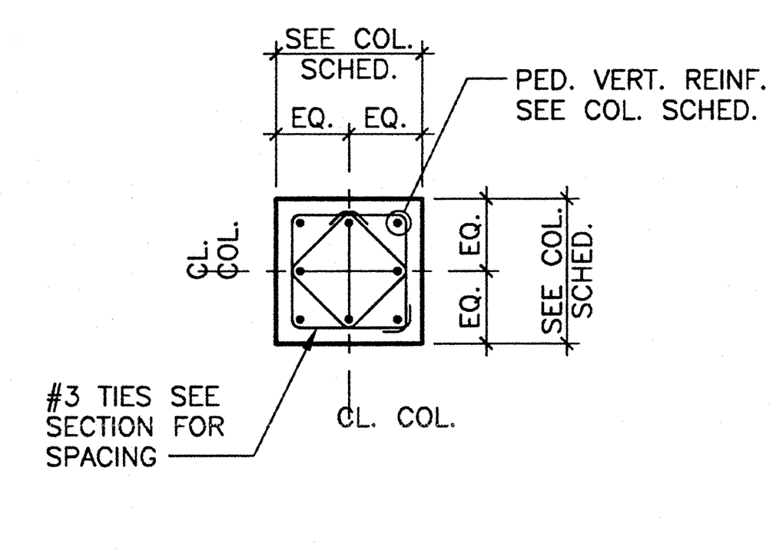
1C TYPICAL DETAIL - COLUMN DOUBLER PLATES @ LATERAL MOMENT CONNECTIONS
NO SCALE SS401

DOUBLER PLATE SCHEDULE			
LEVEL	COLUMN LOCATION	PL. THKNS.	WELD DETAIL
MAIN ROOF	G.5−22.1, G.5−22.8, H.6−21.4 & K.4−21.4	1/2"	A
	J.9−20.9	1/2"	B
	H.6−20.3	3/4"	B
PENTHOUSE ROOF	J.9−18.9, J.9−20.9, K.6−18.9 & K.6−20.9	1/2"	A

NOTES:
1. DOUBLER PLATE SHALL EXTEND PAST COLUMN STIFFENER PLATES A MINIMUM OF 2.5 TIMES THE 'k' VALUE OF THE COLUMN.
2. DOUBLER PLATE TO BE ASTM A572, GRADE 50.
3. SPECIFY ALL 'AWS' WELD DESIGNATIONS IN SHOP DWG.

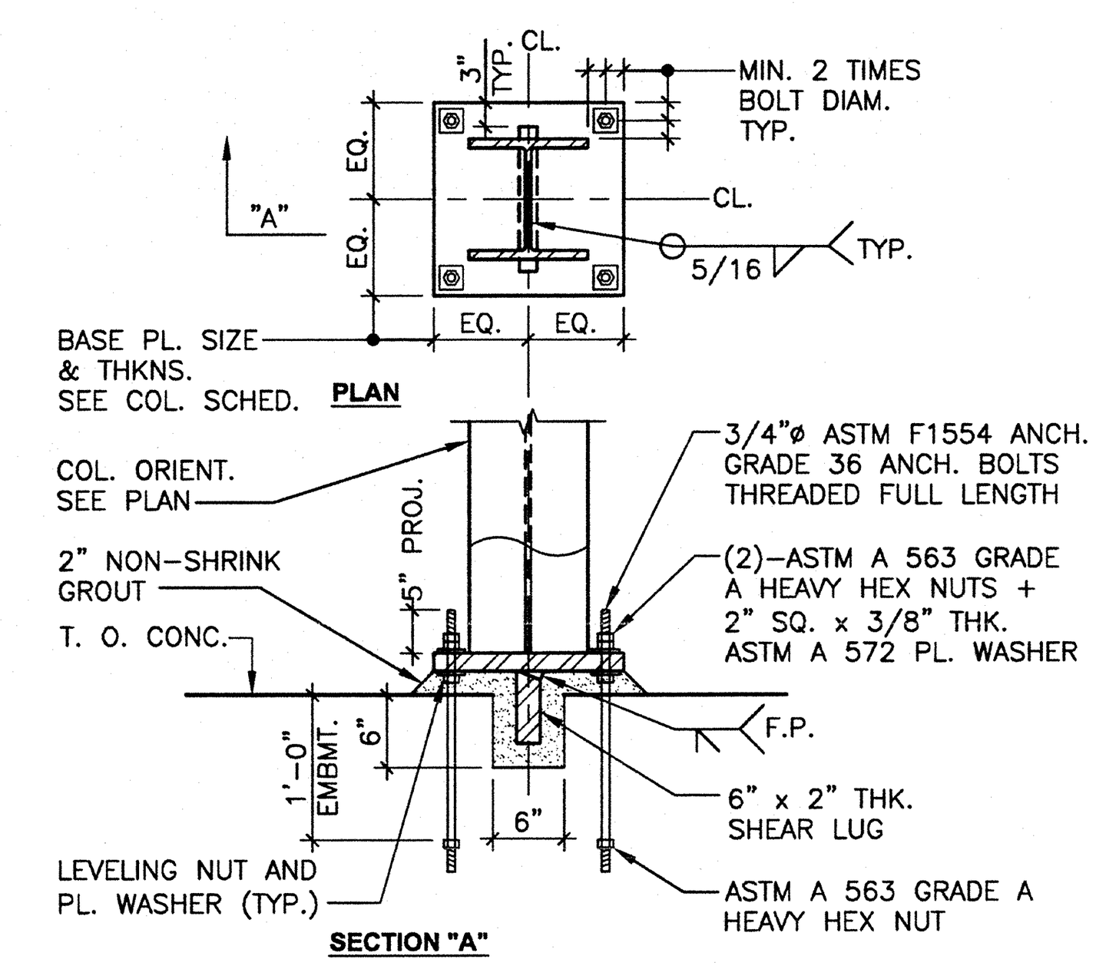


4C TYPICAL DETAIL - COMBINED FOOTING SECTION
NO SCALE SS401

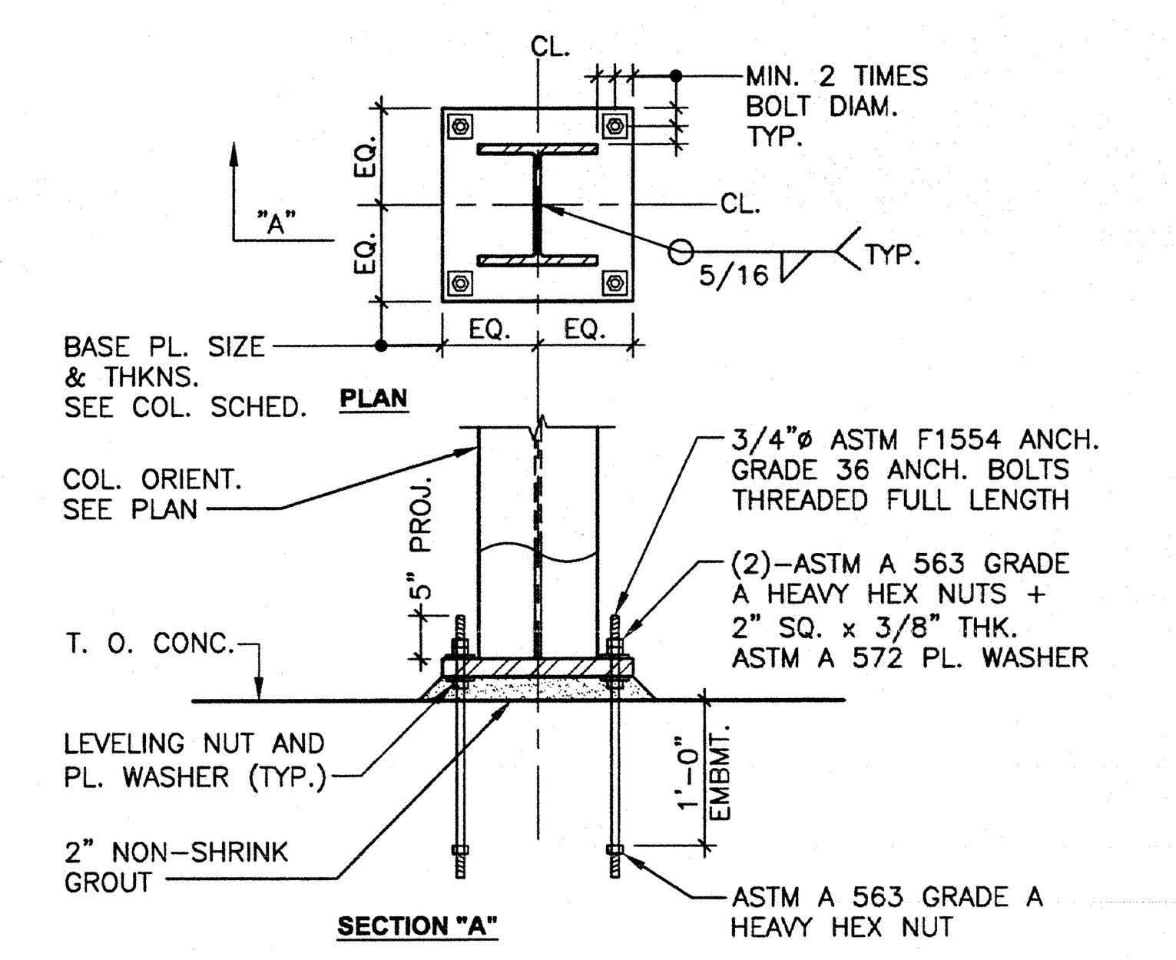


NOTE: FOR PEDESTAL ORIENTATION SEE PLAN

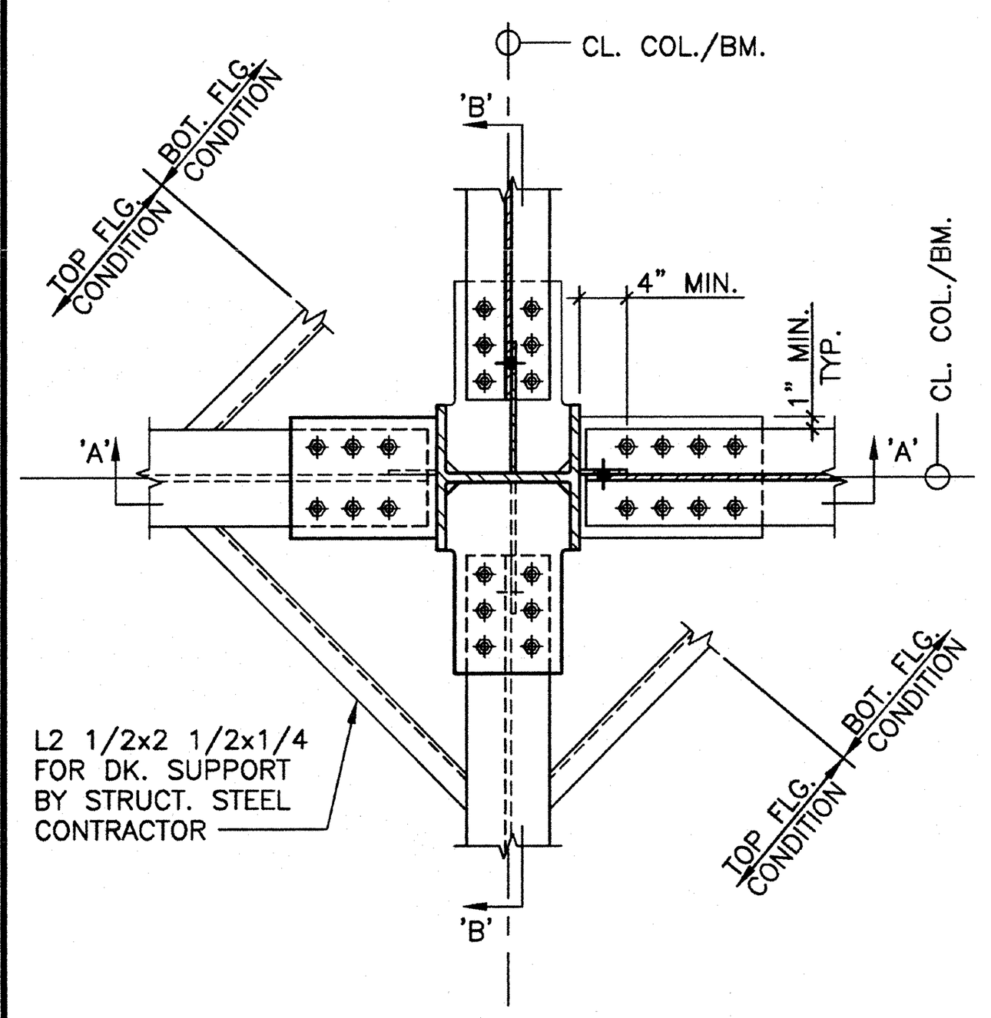
6C TIE/PEDESTAL DETAIL 'A'
NO SCALE SS401



7C TYPICAL DETAIL - COLUMN BASE PLATE TYPE 'C'
NO SCALE SS401



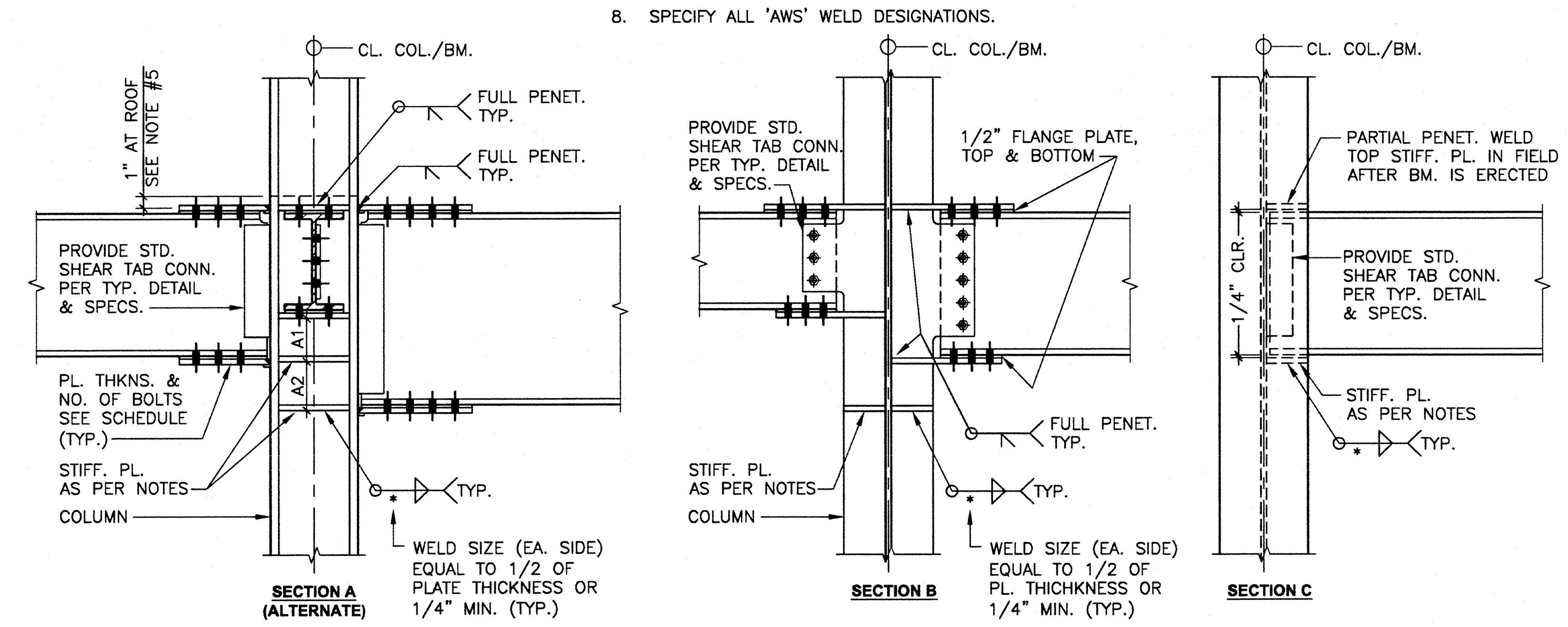
9C TYPICAL DETAIL - COLUMN BASE PLATE TYPE 'A'
NO SCALE SS401



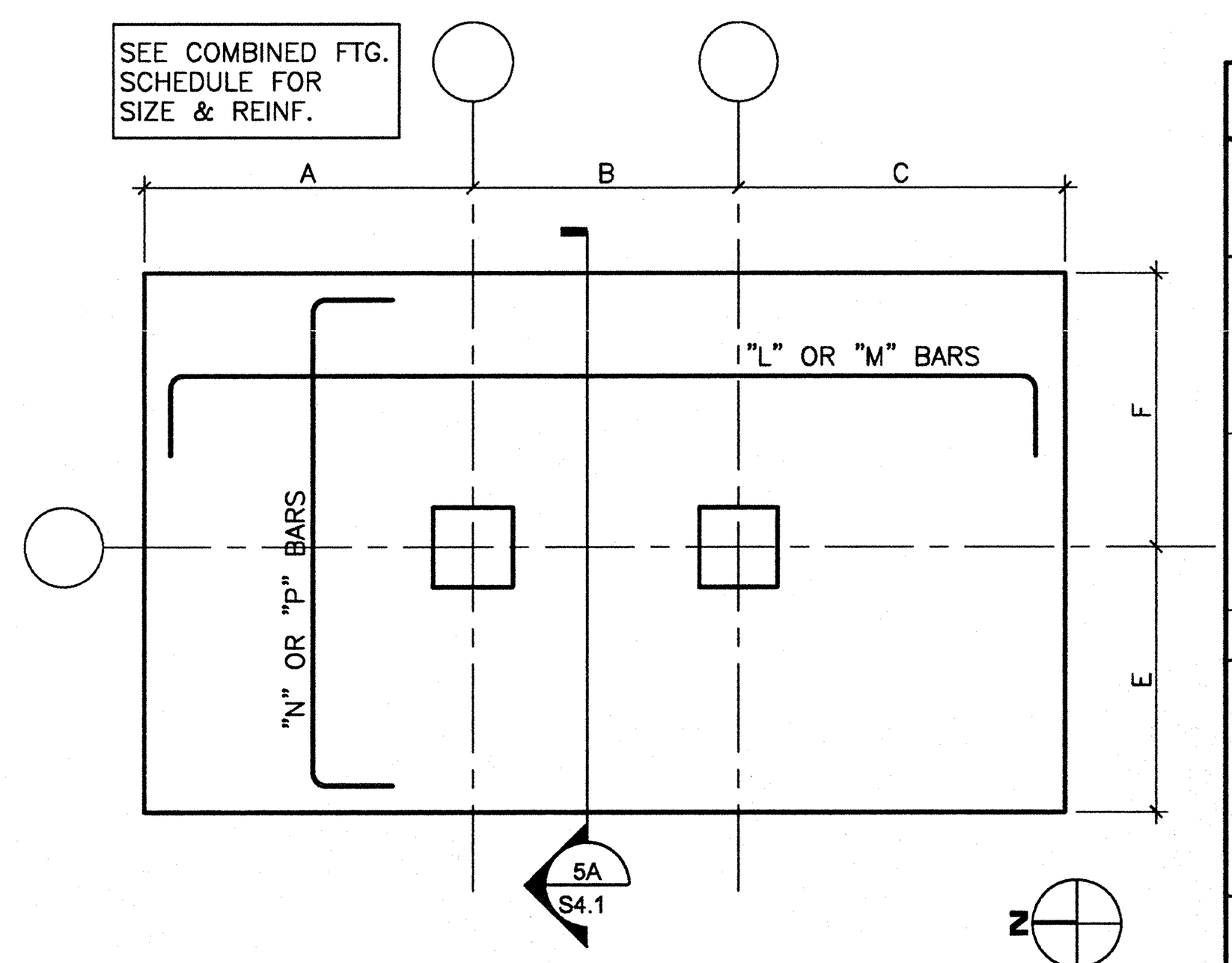
GRAVITY & LATERAL MOMENT CONNECTION NOTES:

1. PROVIDE LATERAL MOMENT CONNECTIONS AT ALL BEAM / COLUMN JOINTS INDICATED THUS ─◀ IN PLAN.
2. WHEN DIMENSION A1 OR A2 IS GREATER THAN 3 INCHES PROVIDE A FITTED STIFF. PLATE BETWEEN THE COLUMN FLANGES. THE THICKNESS OF THE STIFF. PLATE SHALL EQUAL THE THICKNESS OF THE BOTTOM LATERAL MOMENT PLATE FOR THE BEAM FRAMING TO COL. FLANGE.
3. WHEN DIMENSION A1 IS GREATER THAN 3 INCHES BUT A2 IS EQUAL TO OR LESS THAN 3 INCHES PROVIDE A FITTED STIFF. PL. AT THE LOWER BOTTOM LATERAL MOMENT PL. ONLY.
4. THE STEEL DETAILER SHALL COORDINATE NOTES 2 AND 3 ABOVE WITH THE FRAMING CONDITION AT EACH COLUMN. STIFFENER PLATE REQUIREMENTS ARE TO BE CONSIDERED INDEPENDENTLY FOR EACH SIDE OF THE COLUMN WEB. AT THE CONDITION WHERE NO BEAM FRAMES TO THE COLUMN WEB ON EITHER ONE OR BOTH SIDES, FITTED STIFFENER PLATES SHALL BE PROVIDED AT BOTH THE TOP AND BOTTOM FLANGES.
5. AT ROOF LEVELS, EXTEND THE COLUMN SHAFT A MINIMUM OF 1 INCH ABOVE THE HIGHEST BM. FLANGE FRAMING TO THE COLUMN TO ACCOMMODATE MOMENT CONNECTION PLATE WELD.
6. WHEN BEAM FRAMING INTO COL. WEB DOES NOT RECEIVE A MOMENT CONNECTION PROVIDE STANDARD DOUBLE ANGLE SHEAR CONNECTION AND COORDINATE WITH STIFF. PLATES REQ'D. FOR BEAM FRAMING INTO COL. FLANGES. SEE SECTION C.
7. TRIM METAL DECKING AT LATERAL MOMENT CONNECTIONS AND COLUMN, AS REQUIRED, TO LAY FLAT. TYPICAL ALL LEVELS INCLUDING ROOF.
8. SPECIFY ALL 'AWS' WELD DESIGNATIONS.

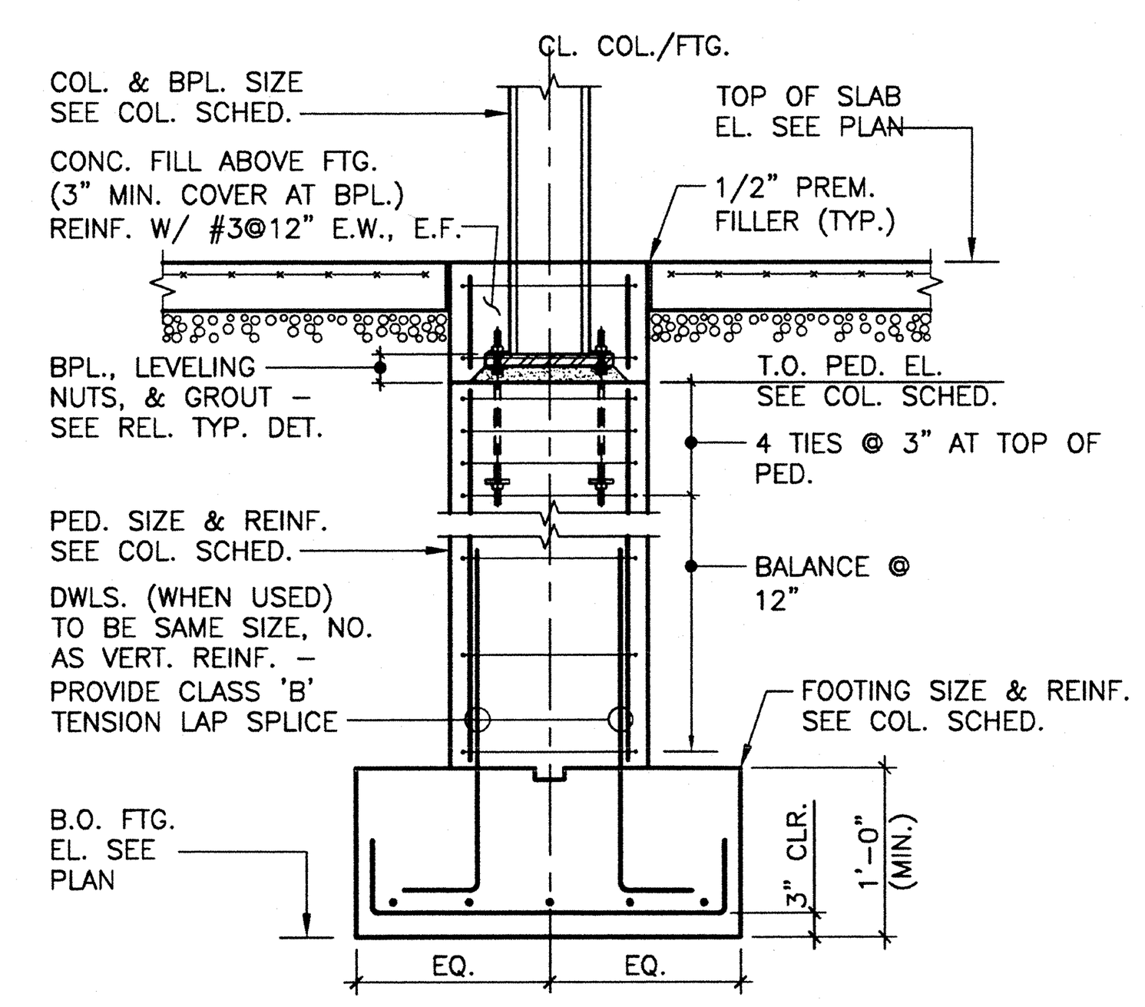
MOMENT CONNECTION SCHEDULE			
BEAM SIZE	PL. THKNS.	# OF BOLTS	BOLT TYPE
W14	3/4"	8	1" ø A490-SC
W16	1 1/4"	14	1" ø A490-SC
W18	1"	10	1" ø A490-SC
W21	1"	14	1" ø A490-SC
W24	3/4"	6	1" ø A490-SC



1F TYPICAL DETAIL - LATERAL MOMENT CONNECTION ─◀
NO SCALE SS401



4D TYPICAL PLAN DETAIL - COMBINED FOOTING
NO SCALE SS401



4F TYPICAL DETAIL - COLUMN FOOTING WITH PEDESTAL
NO SCALE SS401

COLUMN SCHEDULE

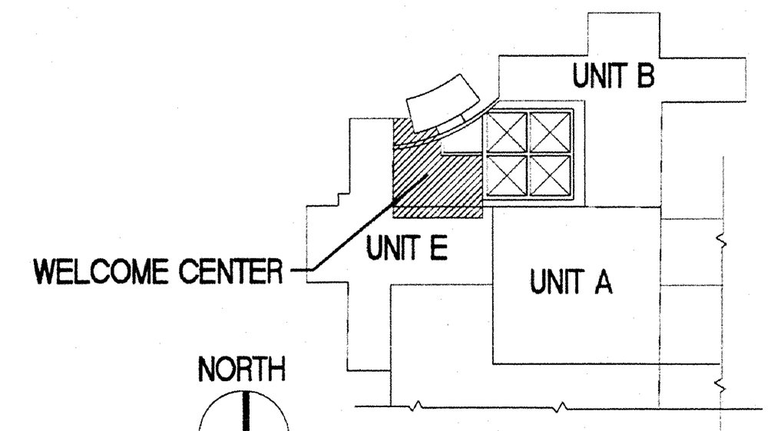
COLUMN NUMBER	G.5	G.5	G.9	G.9	G.9	H.6	H.6	H.6	H.6	H.6	J.8	J.9	J.9	J.9	J.9	J.9	K.4	K.4	K.4	K.6	K.6	K.6
COLUMN SUPPORT	22.1	22.8	20.8	21.4	22.1	18.9	20.3	21.4	22.1	22.8	22.8	18.9	20.3	20.9	21.4	22.1	21.4	22.1	22.8	18.9	20.3	20.9
02 PENTHOUSE LEVEL																						
01 ROOF LEVEL	W8x40	W8x40	W8x40	W8x40	W8x40	W8x40	W8x40	W8x40	W8x40	W8x40	W8x40	W8x40	W8x40	W8x40	W8x40	W8x40	W8x40	W8x40	W8x40	W8x40	W8x40	W8x40
BASE PLATE SIZE N.-S.	14"	14"	14"	14"	14"	14"	14"	14"	14"	14"	14"	14"	14"	14"	14"	14"	14"	14"	14"	14"	14"	14"
BASE PLATE SIZE E.-W.	14"	14"	14"	14"	14"	14"	14"	14"	14"	14"	14"	14"	14"	14"	14"	14"	14"	14"	14"	14"	14"	14"
BASE PLATE THICKNESS	1 1/4"	1 1/4"	1 1/4"	1 1/4"	1 1/4"	1 1/4"	1 1/4"	1 1/4"	1 1/4"	1 1/4"	1 1/4"	1 1/4"	1 1/4"	1 1/4"	1 1/4"	1 1/4"	1 1/4"	1 1/4"	1 1/4"	1 1/4"	1 1/4"	1 1/4"
BASE PLATE TYPE	D	D	D	D	D	D	B	A	A	B	B	C	A	A	A	A	A	B	D	D	B	A
PEDESTAL TOP ELEV.			−2'−0"	−2'−0"							−1'−0"	−1'−0"	−1'−0"	−1'−0"	−1'−0"	−1'−0"	−1'−0"	−1'−0"	−1'−0"	−1'−0"	−1'−0"	−1'−0"
PEDESTAL SIZE N.-S.			34"	31"							22"	22"	22"	22"	22"	22"	22"	22"	22"	22"	22"	22"
PEDESTAL SIZE E.-W.			22"	18"							22"	22"	22"	22"	22"	22"	22"	22"	22"	22"	22"	22"
PEDESTAL VERT. REINF.			8−#9	8−#8							8−#8	8−#8	8−#8	8−#8	8−#8	8−#8	8−#8	8−#8	8−#8	8−#8	8−#8	8−#8
PEDESTAL TIES			#3	#3							#3	#3	#3	#3	#3	#3	#3	#3	#3	#3	#3	#3
PEDESTAL TIE DETAIL			B	B							A	A	A	A	A	A	A	A	A	A	A	A
FOOTING SIZE N.-S.	5'−0"	5'−0"	5'−0"	5'−0"	6'−0"	7'−5"	11'−11"	6'−0"	6'−0"	6'−0"	SEE COMBINED FTG. SCHED.	SEE COMBINED FTG. SCHED.	8'−0"	6'−0"	6'−0"	SEE COMBINED FTG. SCHED.	6'−0"	SEE COMBINED FTG. SCHED.	SEE COMBINED FTG. SCHED.	SEE COMBINED FTG. SCHED.	7'−0"	7'−0"
FOOTING SIZE E.-W.	5'−0"	5'−0"	5'−0"	5'−0"	6'−0"	8'−6"	12'−0"	6'−0"	6'−0"	6'−0"			8'−0"	6'−0"	6'−0"		6'−0"				9'−0"	7'−0"
FOOTING THICKNESS	1'−6"	1'−6"	1'−0"	1'−0"	1'−6"	1'−6"	1'−6"	1'−6"	1'−6"	1'−6"			1'−6"	1'−6"	1'−6"		1'−6"				1'−0"	1'−0"
FOOTING REINF. N-S	5−#6 T.&B.	5−#6 T.&B.	5−#6 T.&B.	5−#6 T.&B.	6−#6 B.	8−#6 T.&B.	12−#6 T.&B.	6−#6 B.	6−#6 B.	6−#6 B.			8−#6 B.	6−#6 B.	6−#6 B.		5−#6 T.&B.				8−#6 B.	7−#6 B.
FOOTING REINF. E-W	5−#6 T.&B.	5−#6 T.&B.	5−#6 T.&B.	5−#6 T.&B.	6−#6 B.	6−#6 T.&B.	12−#6 T.&B.	6−#6 B.	6−#6 B.	6−#6 B.			8−#6 B.	6−#6 B.	6−#6 B.		5−#6 T.&B.				7−#6 B.	7−#6 B.

COLUMN SUPPORT: 22.1 22.8 20.8 21.4 22.1 18.9 20.3 21.4 22.1 22.8 22.8 18.9 20.3 20.9 21.4 22.1 21.4 22.1 22.8 18.9 20.3 20.9
COLUMN NUMBER: G.5 G.9 H.6 J.8 J.9 K.4 K.6

COMBINED FOOTING SCHEDULE

COLUMN LOCATIONS	DIMENSIONS A	B	C	D	E	F	REINFORCING L	M	N	P
J.9−18.9 & K.6−18.9	3'−6"	13'−0"	3'−6"	1'−0"	2'−9"	2'−9"	5−#6	7−#6	N.A.	#6@12"
J.9−22.1 & K.4−22.1	4'−0"	9'−6"	1'−0"	1'−0"	4'−6"	4'−6"	7−#6	7−#6	N.A.	#6@12"
J.8−22.8 & K.4−22.8	2'−6"	11'−6"	2'−6"	1'−0"	1'−0"	3'−0"	5−#6	5−#6	N.A.	#6@12"

FOR COMBINED FOOTING TYPICAL DETAIL SEE 5 & 5A/S4.1



KEY PLAN

Revisions	Date
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Drawing Title
COLUMN SCHEDULE AND DETAILS

Approved: Project Director

Project Title
OIF / OEF WELCOME CENTER
DEPARTMENT OF VETERANS AFFAIRS
VAMC

Location Veterans Affairs Medical Center
50 Irving Street NW Washington DC

Date 4-30-2013 | Checked JRM | Drawn BEC

Project Number 688-334 OIF/OEF

Building Number

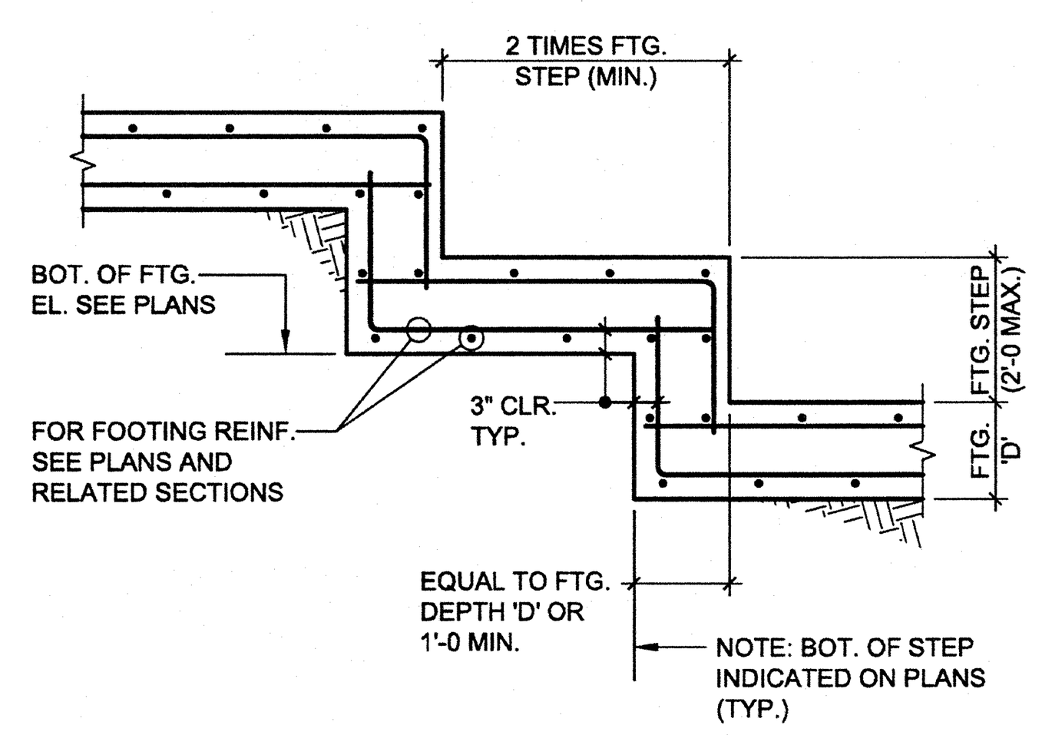
Drawing Number SS401

Office of Construction and Facilities Management

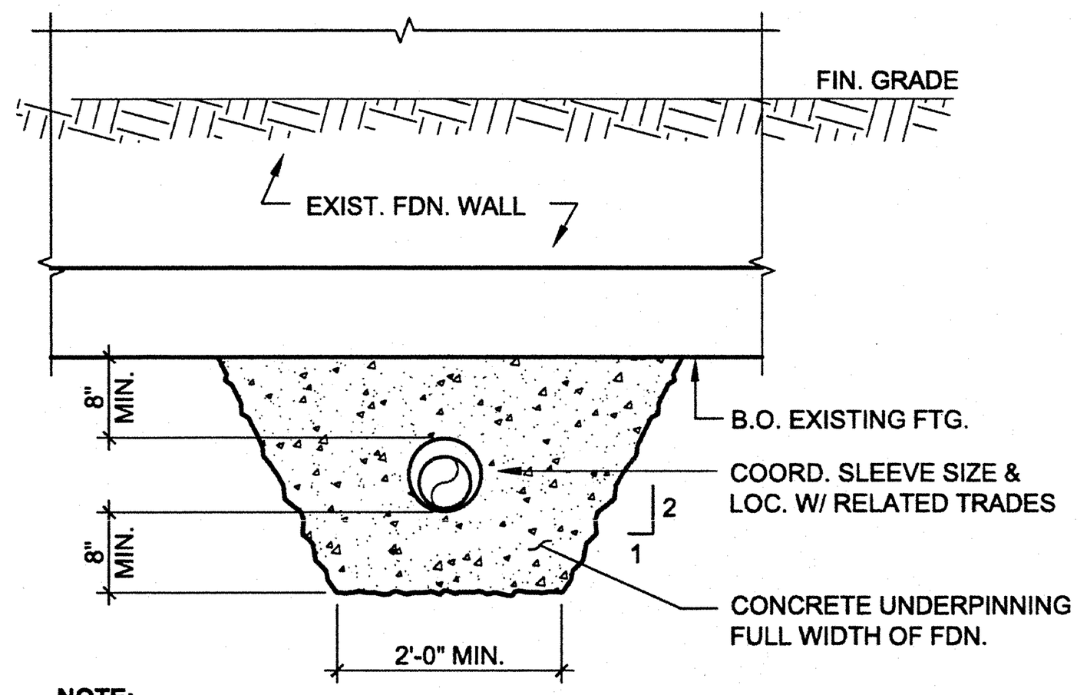
Department of Veterans Affairs

VA FORM 08-6231

NOTE:
1. COORDINATE STEP LOCATION WITH PLANS. FIRST STEP FROM COLUMN FOOTING MINIMUM 2'-0.



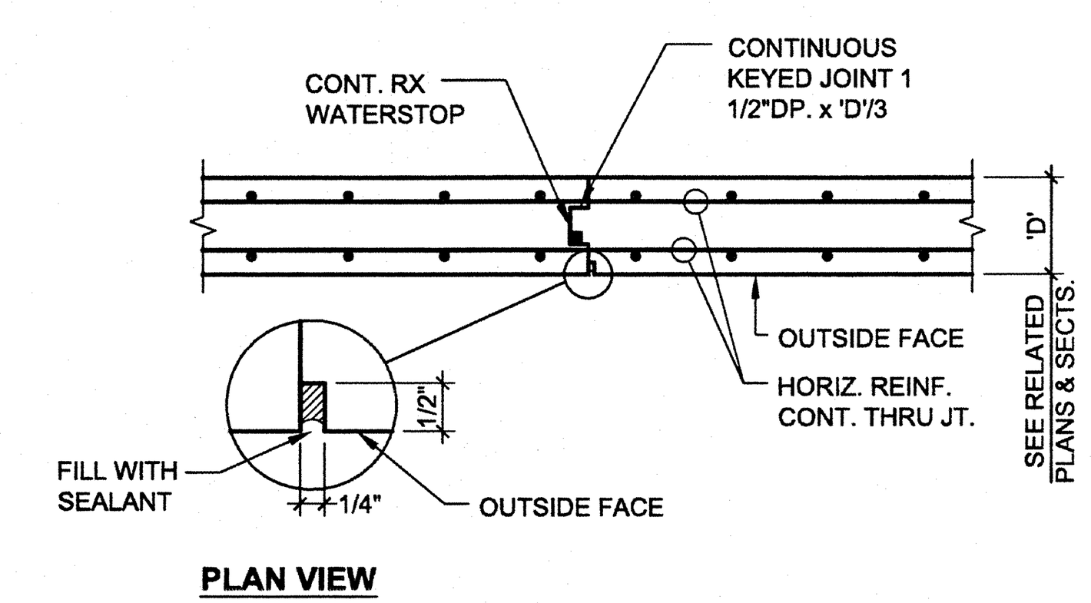
5B TYPICAL DETAIL - STEPPED FOOTING WITH TOP AND BOTTOM REINFORCING
NO SCALE SS501



NOTE:
THE CONTRACTOR SHALL PROVIDE ALL TEMPORARY SHORING AND PROTECTION OF THE EXISTING STRUCTURAL ELEMENTS TO ENSURE THE STRUCTURAL INTEGRITY OF THE EXISTING BUILDING DURING THE CONSTRUCTION OF THE PIPE UNDERPINNING. PROVIDE MINIMUM SCHEDULE 40 STANDARD STEEL PIPE SLEEVE

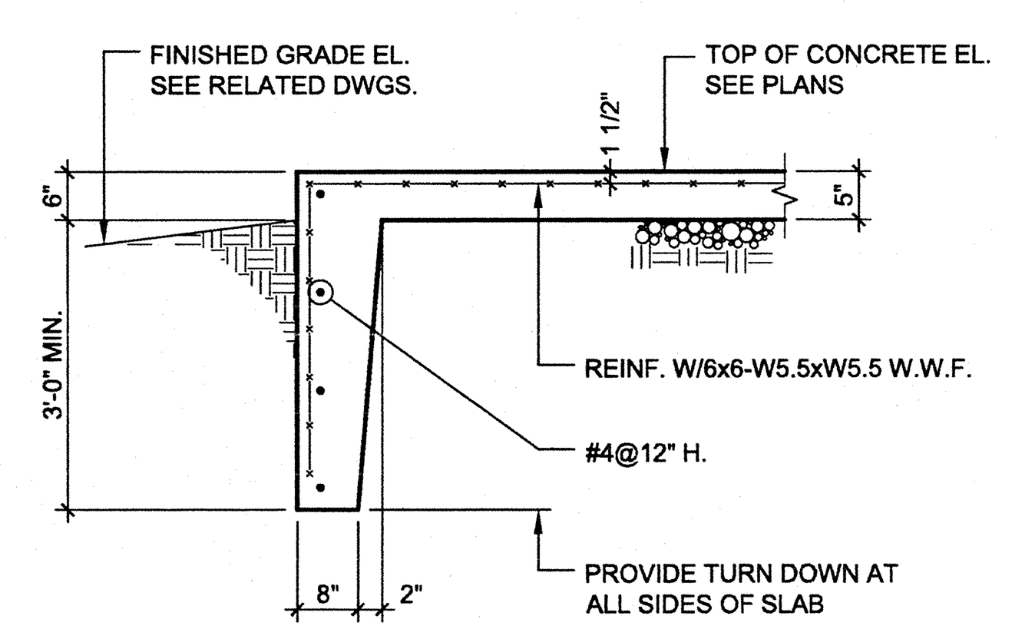
3B TYPICAL DETAIL - PIPE/CONDUIT SLEEVE UNDER EXISTING FOUNDATION
NO SCALE SS501

NOTE:
PROVIDE CONSTRUCTION JOINTS IN CONCRETE WALLS AT 80'-0 C./C. MAX. SPACING. CONSTRUCTION JOINTS SHALL BE LOCATED WITHIN THE MIDDLE THIRD OF SPAN WHERE WALLS ARE EXPOSED TO THE EXTERIOR. SUBMIT A DRAWING OF ALL PROPOSED WALL JOINTS FOR ARCHITECTS REVIEW.



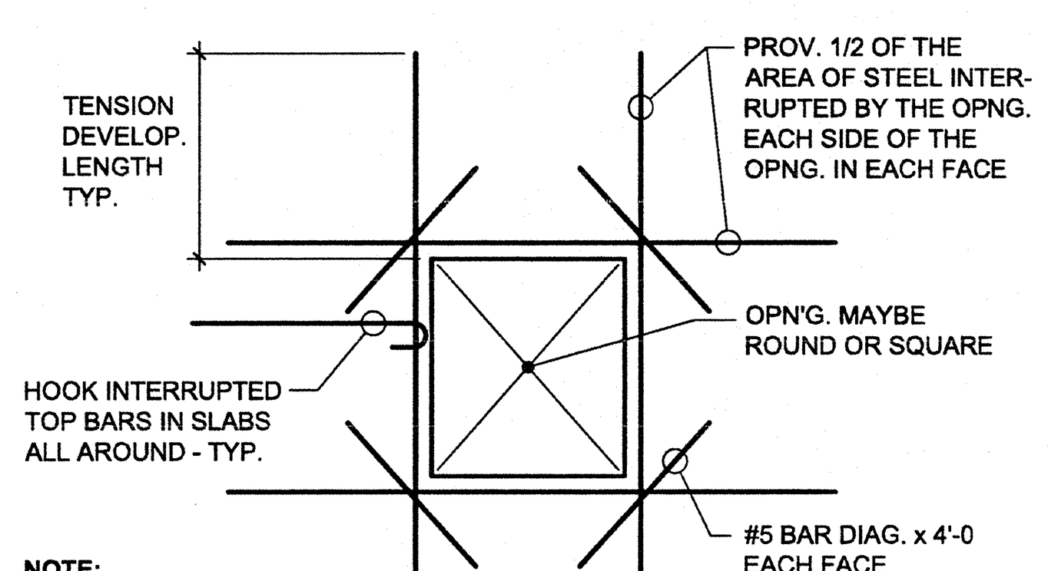
5C TYPICAL DETAIL - CONCRETE WALL CONSTRUCTION JOINT
NO SCALE SS501

STANDARD HOOK TENSION DEVELOPMENT LENGTHS (UNCOATED BARS) (IN.)						
BAR SIZE	3,000 PSI CONCRETE		4,000 PSI CONCRETE		5,000 PSI CONCRETE	
	NW CONC.	LW CONC.	NW CONC.	LW CONC.	NW CONC.	LW CONC.
#3	9	11	8	10	7	9
#4	11	15	10	13	9	12
#5	14	18	12	16	11	14
#6	17	22	15	19	13	17
#7	20	25	17	22	15	20
#8	22	29	19	25	17	23
#9	25	33	22	28	20	25
#10	28	36	24	31	22	28
#11	31	40	27	34	24	31



NOTES:
1. COORDINATE SLAB LOCATION AND GEOMETRY WITH PLANS.
2. SUBSOIL BELOW PAD TO BE PREPARED AS REQUIRED TO SUSTAIN A MINIMUM ALLOWABLE BEARING PRESSURE OF 2000 PSF.
3. SLAB TO BE PLACED ON A MINIMUM 4" CRUSHED STONE SUBBASE.

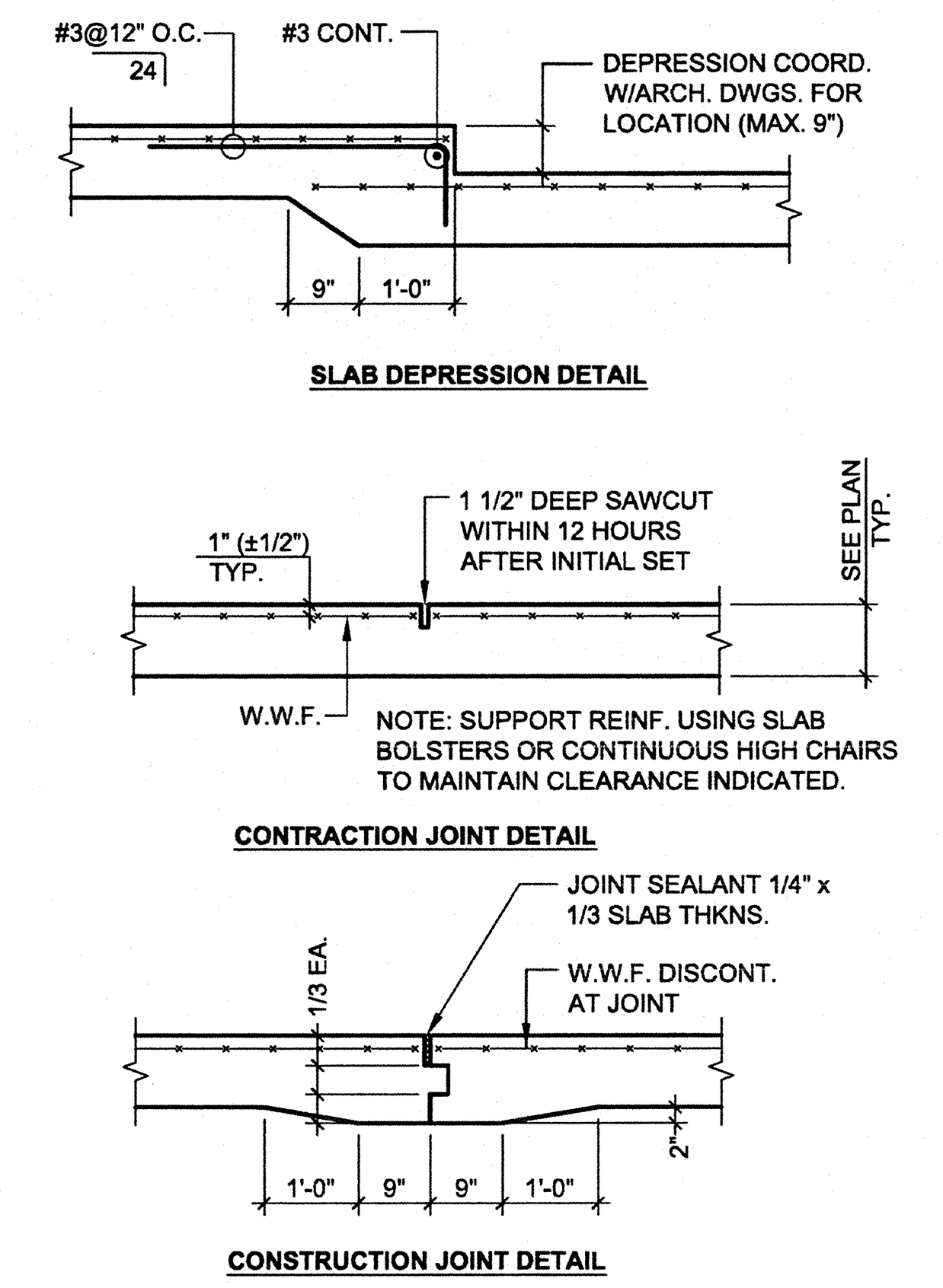
2C TYPICAL DETAIL - EXTERIOR TURNED DOWN SLAB
NO SCALE SS501



NOTE:
INTERRUPTED BARS AT OPNG. WOULD BE TOP AND/OR BOT. BARS IN SLABS OR VERT. AND/OR HORIZ. BARS IN WALLS (NOT SHOWN FOR CLARITY)

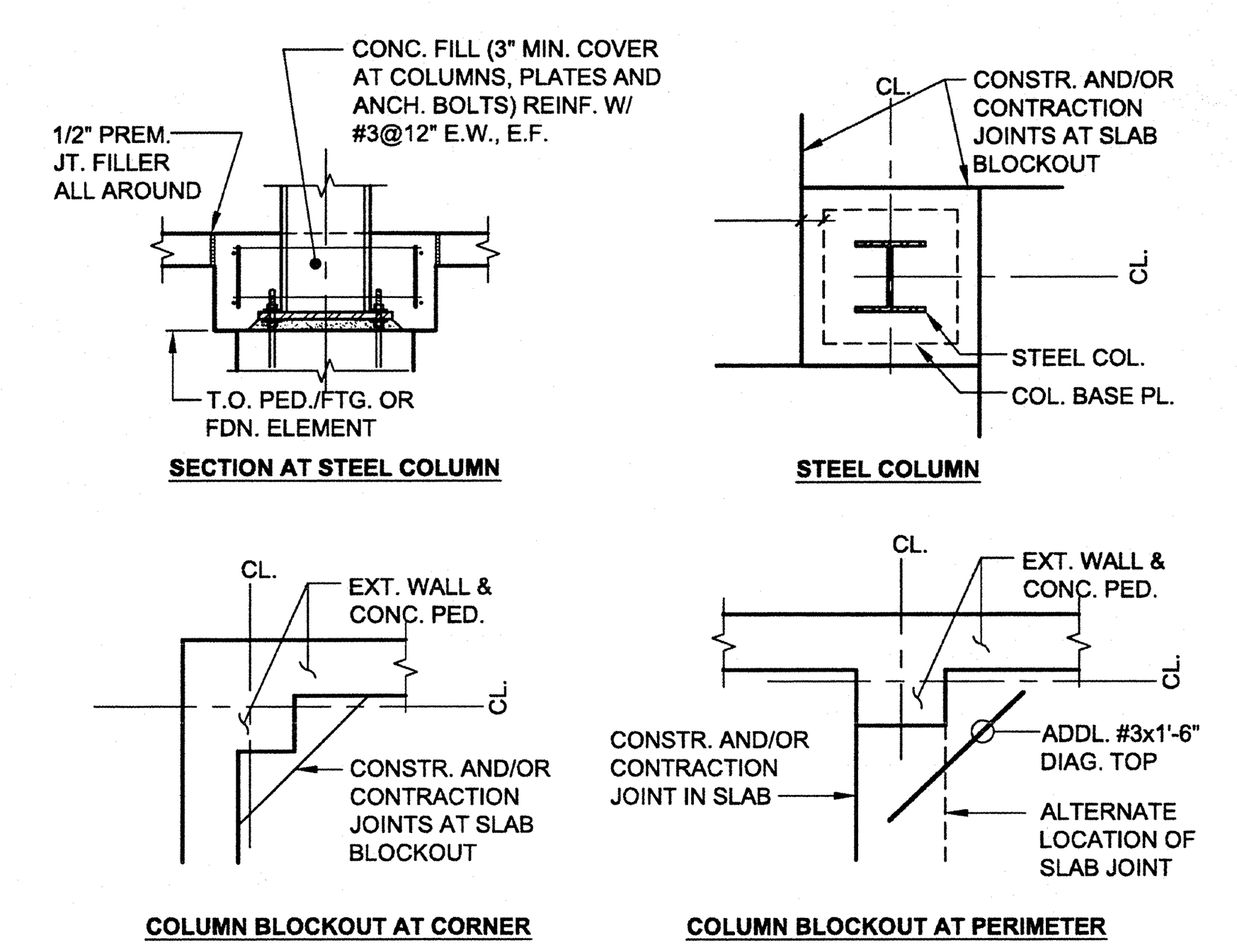
NOTE:
ADDTL. REINF. AS SHOWN REQD. AT OPNGS. 12" SQUARE OR 12" IN DIAM. OR LARGER

3C TYPICAL DETAIL - REINF. AT WALL OR SLAB OPENINGS
NO SCALE SS501



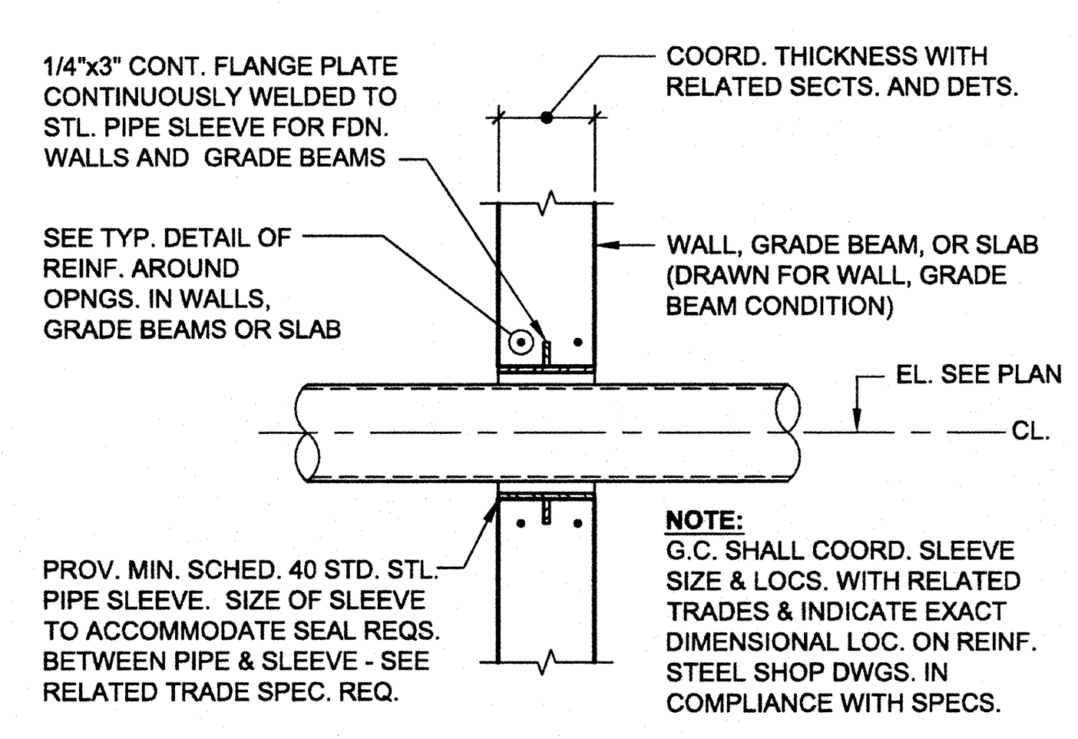
6C TYPICAL DETAIL - SLAB ON GRADE
NO SCALE SS501

NOTE:
CONCRETE CONTRACTOR TO COORDINATE BLOCK OUT DIMENSIONS W/ STEEL CONTRACTOR PRIOR TO POURING CONCRETE. BLOCKOUTS WILL BE LARGER AT CROSS BRACING LOCATIONS.



8C TYPICAL DETAIL - JOINTS AT COLUMN FOR SLAB ON GRADE
NO SCALE SS501

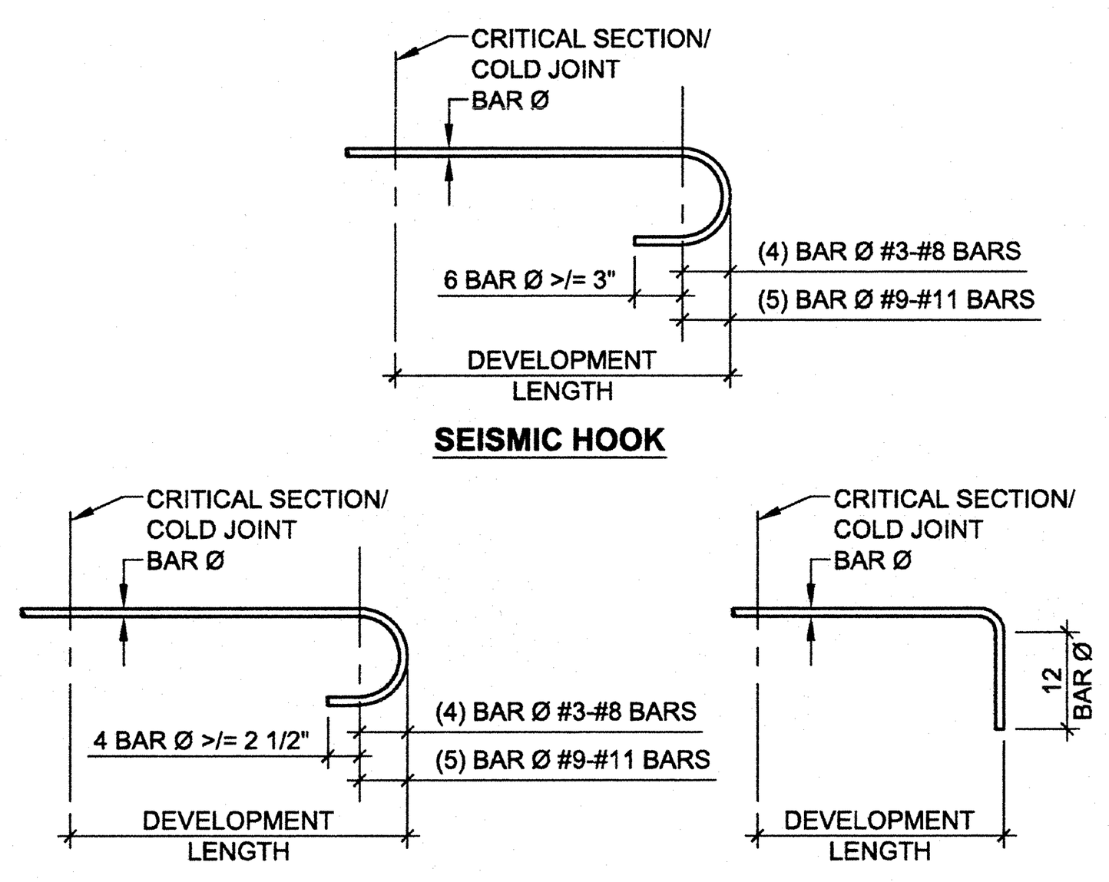
TENSION DEVELOPMENT LENGTHS (UNCOATED BARS)												
BAR SIZE	3,000 PSI CONCRETE				4,000 PSI CONCRETE				5,000 PSI CONCRETE			
	L_d PER SPACING AND COVER				L_d PER SPACING AND COVER				L_d PER SPACING AND COVER			
	CASE 1		CASE 2		CASE 1		CASE 2		CASE 1		CASE 2	
	TOP BARS	OTHERS	TOP BARS	OTHERS	TOP BARS	OTHERS	TOP BARS	OTHERS	TOP BARS	OTHERS	TOP BARS	OTHERS
#3	22	17	32	25	19	15	28	22	17	13	25	19
#4	29	22	43	33	25	19	37	29	22	17	33	26
#5	36	28	54	41	31	24	47	36	28	22	42	32
#6	43	33	64	50	37	29	56	43	33	26	50	38
#7	63	48	94	72	54	42	81	63	49	37	73	56
#8	72	55	107	82	62	48	93	71	55	43	83	64
#9	81	62	121	93	70	54	105	81	63	48	94	72
#10	91	70	136	105	79	61	118	91	70	54	105	81
#11	101	78	151	116	87	67	131	101	78	60	117	90



PROV. MIN. SCHED. 40 STD. STL. PIPE SLEEVE. SIZE OF SLEEVE TO ACCOMMODATE SEAL REQS. BETWEEN PIPE & SLEEVE - SEE RELATED TRADE SPEC. REQ.

NOTE:
G.C. SHALL COORD. SLEEVE SIZE & LOCS. WITH RELATED TRADES & INDICATE EXACT DIMENSIONAL LOC. ON REINF. STEEL SHOP DWGS. IN COMPLIANCE WITH SPECS.

3E TYPICAL DETAIL - PIPE/CONDUIT SLEEVE THRU FOUNDATION WALL GRADE BEAM OR SLAB
NO SCALE SS501



5E TYPICAL DETAIL - STD. HOOK DEVELOPMENT LENGTHS
NO SCALE SS501

TENSION SPLICE LENGTHS (UNCOATED BARS)												
BAR SIZE	3,000 PSI CONCRETE				4,000 PSI CONCRETE				5,000 PSI CONCRETE			
	LAP LENGTH PER SPACING AND COVER				LAP LENGTH PER SPACING AND COVER				LAP LENGTH PER SPACING AND COVER			
	CASE 1		CASE 2		CASE 1		CASE 2		CASE 1		CASE 2	
	TOP BARS	OTHERS	TOP BARS	OTHERS	TOP BARS	OTHERS	TOP BARS	OTHERS	TOP BARS	OTHERS	TOP BARS	OTHERS
#3	28	22	42	32	24	19	36	28	22	17	33	25
#4	37	29	56	43	32	25	48	37	29	22	43	33
#5	47	36	70	54	40	31	60	47	36	28	54	42
#6	56	43	84	64	48	37	72	56	43	33	65	50
#7	81	63	122	94	70	54	106	81	63	49	94	73
#8	93	72	139	107	80	62	121	93	72	55	108	83
#9	105	81	157	121	91	70	136	105	81	63	122	94
#10	118	91	177	136	102	79	153	118	91	70	137	105
#11	131	101	196	151	113	87	170	131	101	78	152	117

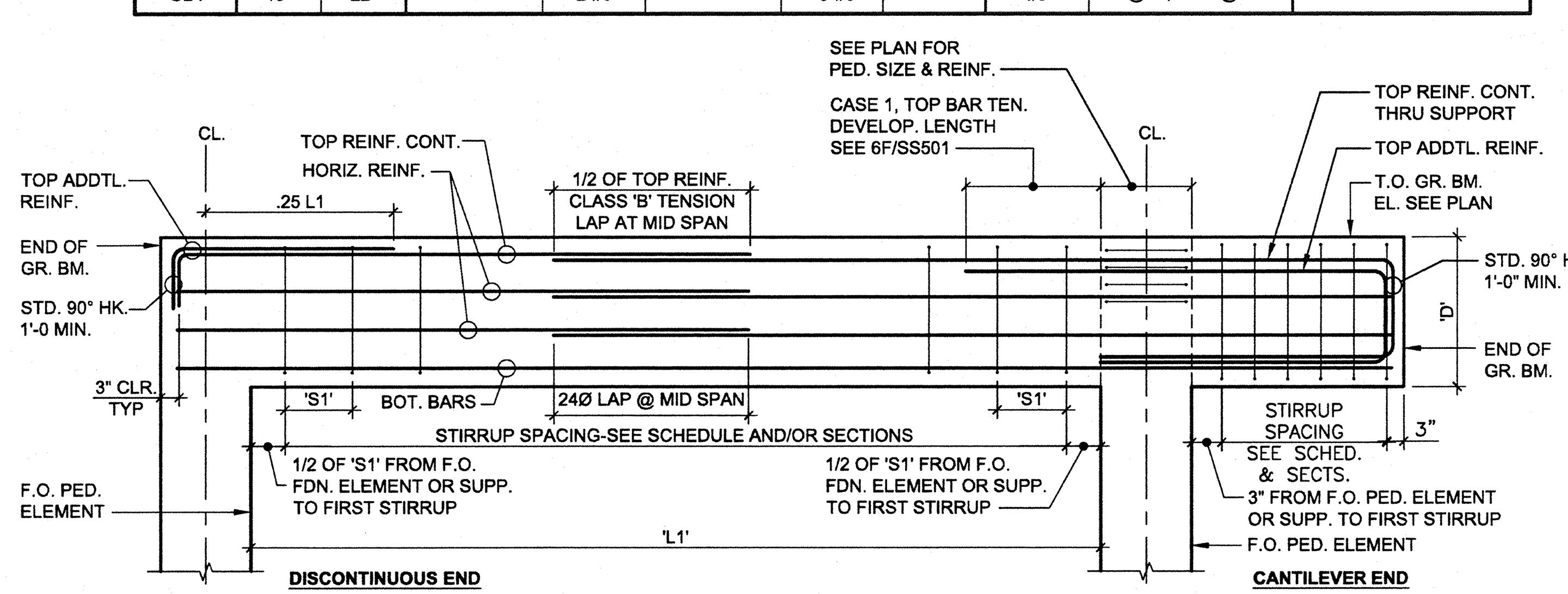
NOTES ON BAR SPLICE

1. CASE 1 - BEAMS AND COLUMNS:
   CONCRETE COVER $\geq$ db, C.-C. BAR SPACING $\geq$ 2db AND WITH STIRRUPS OR TIES THROUGHOUT Ld NOT LESS THAN THE CODE MINIMUM
   OTHER MEMBERS: CONCRETE COVER $\geq$ db, C.-C. BAR SPACING $\geq$ 3db

   CASE 2 BEAMS AND COLUMNS:
   CONCRETE COVER < db AND C.-C. BAR SPACING < 2db
   OTHER MEMBERS: CONCRETE COVER < db OR C.-C. BAR SPACING < 3db
2. VERTICAL COLUMN REINFORCING SHALL BE A CONTINUOUS BAR WHENEVER POSSIBLE, IF SPLICES ARE REQUIRED USE LAP LENGTHS SHOWN AND SUBMIT FOR ENGINEER'S APPROVAL.
3. SPLICES FOR VERTICAL BARS IN WALLS SHALL BE AS SHOWN. SPLICES FOR HORIZONTAL BARS SHALL BE LOCATED:
   (A.) AT PIERS (COLUMNS) FOR INSIDE REBARS
   (B.) IN BETWEEN PIERS FOR OUTSIDE REBARS
4. SPLICES AND DEVELOPMENT LENGTHS FOR EPOXY COATED REBARS SHALL BE INCREASED BY 20 PERCENT FROM THE VALUES SHOWN IN THE TABLES.
5. ALL SPLICES SHALL BE CLASS B TENSION LAP SPLICES. (UNLESS OTHERWISE NOTED)
6. ALL LENGTHS SHOWN ARE IN INCHS
7. TOP BARS ARE HORIZONTAL BARS WITH MORE THAN 12" OF CONCRETE CAST BELOW BARS.

6F TYPICAL DETAIL - TENSION DEVELOPMENT AND LAP SPLICE LENGTHS FOR UNCOATED BARS
NO SCALE SS501

GRADE BEAM SCHEDULE										
BEAM MARK	SIZE		REINFORCEMENT					STIRRUPS		REMARKS
	W	D	TOP ADDTL. S. OR E.	TOP CONT.	TOP ADDTL. N. OR W.	BOT.	HORIZ. E.F.	SIZE	SPACING EACH END	
GB1	15"	22"	---	2-#6	---	3-#6	---	#3	1 @ 3", BAL. @ 12"	

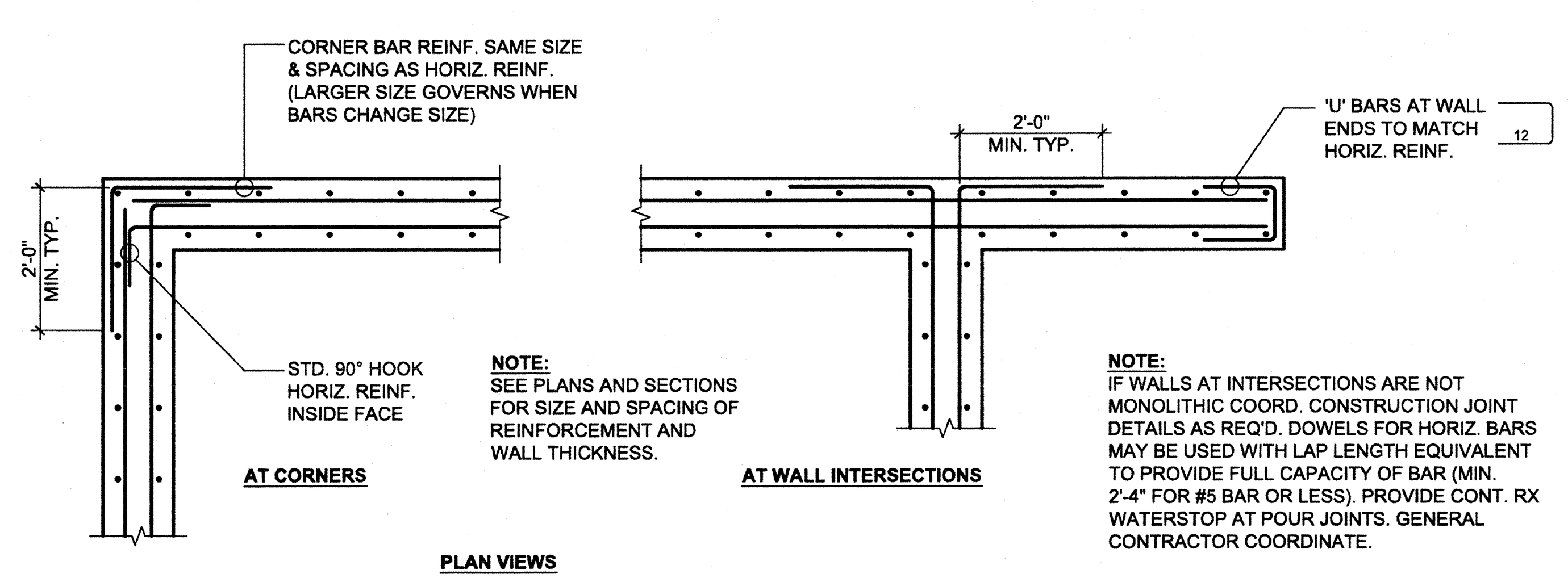


GRADE BEAM CONSTRUCTION JOINT LOCATIONS:

1. CONSTUCTION JOINTS FOR GRADE BEAMS SUPPORTED AT EACH END BY PEDESTALS OR OTHER GRADE BEAMS SHALL BE PLACED AT THE MIDDLE THIRD OF THE SPAN AS REQUIRED. DO NOT PLACE CONSTRUCTION JOINTS AT THE FACE OF PEDESTALS OR GRADE BEAM SUPPORTS. SEE SECTION 16/S5.1 FOR REINFORCEMENT REQUIREMENTS.
2. CONSTRUCTION JOINTS SHALL NOT OCCUR WITHIN THE LENGTH OF THE REBAR LAP SPLICES. SPLICES SHALL BE LOCATED ON EITHER SIDE OF JOINT.
3. ALL CANTILEVER GRADE BEAMS SHALL BE POURED MONOLITHIC WITH THE SUPPORTING PEDESTAL OR GRADE BEAM. DO NOT PLACE CONSTRUCTION JOINTS AT THE FACE OF PEDESTALS OR GRADE BEAM SUPPORTS.

NOTES:
1. GRADE BEAM REINF. DETAILS SHALL BE COORDINATED WITH RELATED FOUNDATION ELEMENTS.
2. PROVIDE ADDTL. DWLS. FROM FDN. TO GR. BM. (TYP.)
3. FOR GR. BM. REINF. - SEE SCHED. AND/OR DTLS.

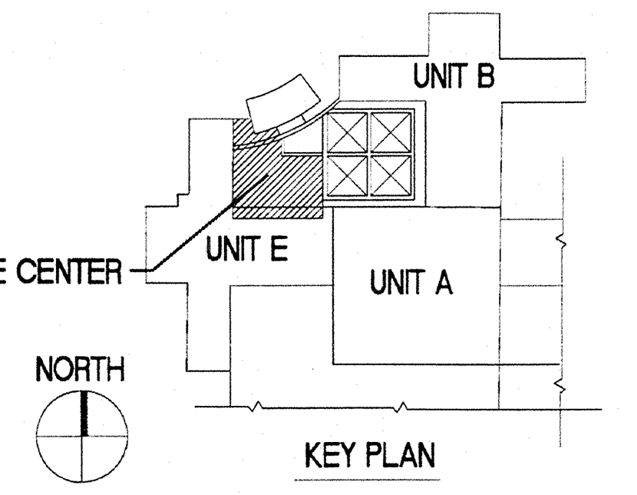
1F TYPICAL DETAIL - GRADE BEAM REINFORCING
NO SCALE SS501



NOTE:
SEE PLANS AND SECTIONS FOR SIZE AND SPACING OF REINFORCEMENT AND WALL THICKNESS.

NOTE:
IF WALLS AT INTERSECTIONS ARE NOT MONOLITHIC COORD. CONSTRUCTION JOINT DETAILS AS REQ'D. DOWELS FOR HORIZ. BARS MAY BE USED WITH LAP LENGTH EQUIVALENT TO PROVIDE FULL CAPACITY OF BAR (MIN. 2'-4" FOR #5 BAR OR LESS). PROVIDE CONT. RX WATERSTOP AT POUR JOINTS. GENERAL CONTRACTOR COORDINATE.

3F TYPICAL DETAIL - REINFORCING FOR CONCRETE WALLS
NO SCALE SS501



KEY PLAN

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Drawing Title: **TYPICAL DETAILS**

Approved: Project Director

Project Title: OIF / OEF WELCOME CENTER DEPARTMENT OF VETERANS AFFAIRS VAMC

Location: Veterans Affairs Medical Center, 50 Irving Street NW Washington DC

Date: 4-30-2013 | Checked: JRM | Drawn: BEC

Project Number: 688-334 OIF/OEF

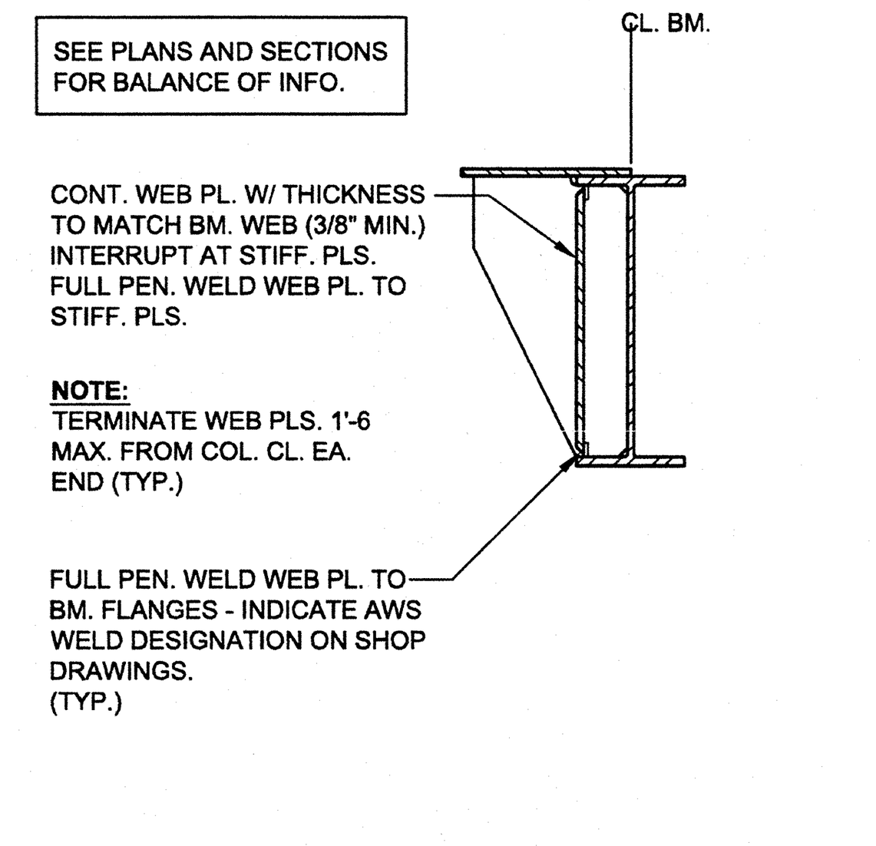
Building Number:

Drawing Number: SS501

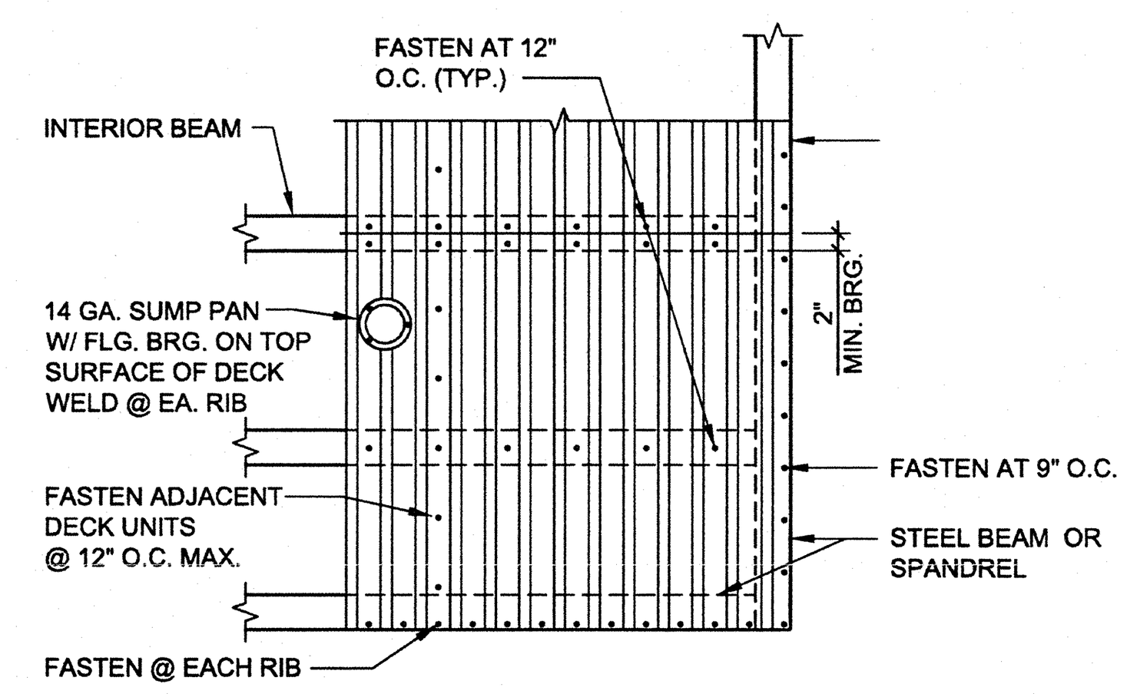
Office of Construction and Facilities Management

Department of Veterans Affairs

VA FORM 08-6231

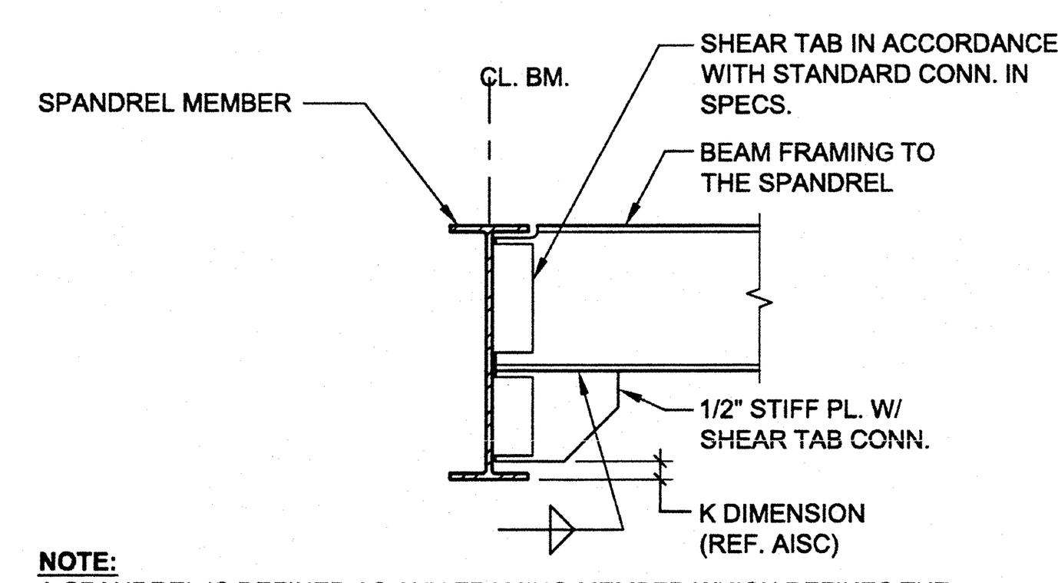


3B TYPICAL DETAIL - BEAM WEB PLATE
NO SCALE SS502



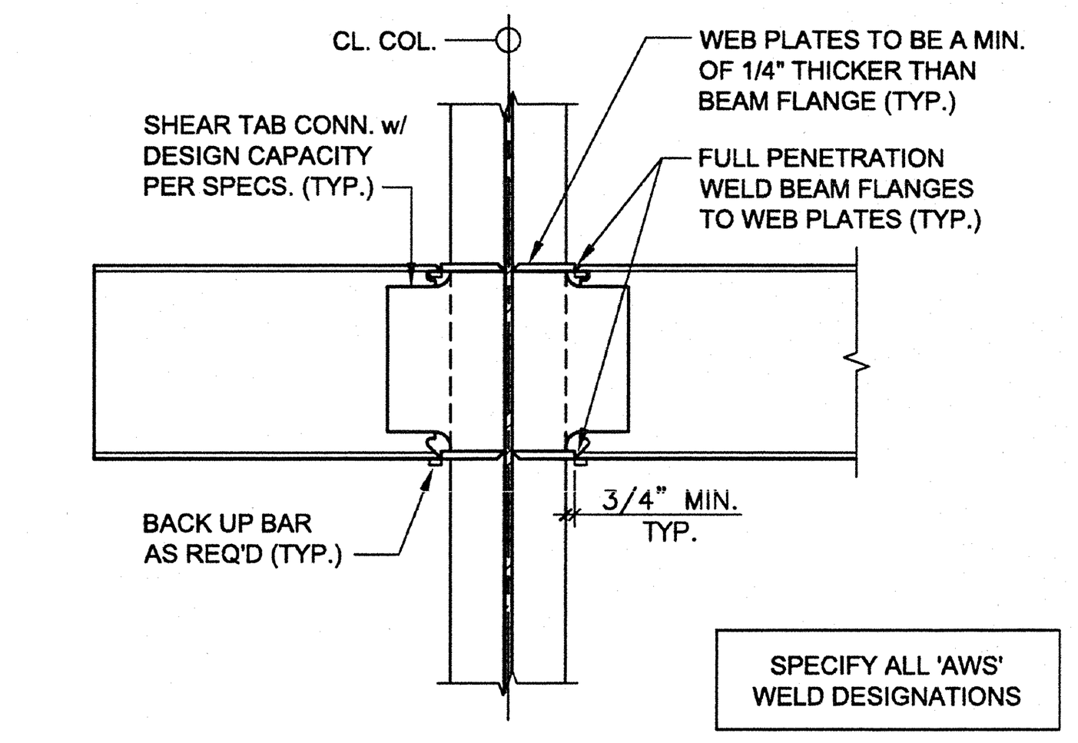
NOTE:
1. ALL FASTENING OF METAL DECK TO STEEL SHALL BE MADE WITH POWDER DRIVEN FASTENERS OR PUDDLE WELDS. PUDDLE WELDS SHALL BE 5/8 INCH DIAM. FOR FLOOR DECKS, AND 3/4 INCH DIAM. FOR ROOF DECKS. POWDER DRIVEN FASTENERS SHALL HAVE EQUIVALENT OR GREATER WORKING CAPACITY THAN 5/8 INCH DIAM. PUDDLE WELD FOR FLOOR DECKS, AND 3/4 INCH DIAM. FOR ROOF DECKS. SEE SPECIFICATIONS.
2. ALL METAL DECK IS USED FOR DIAPHRAGM ACTION.

5B TYPICAL DETAIL - METAL DECK
NO SCALE SS502

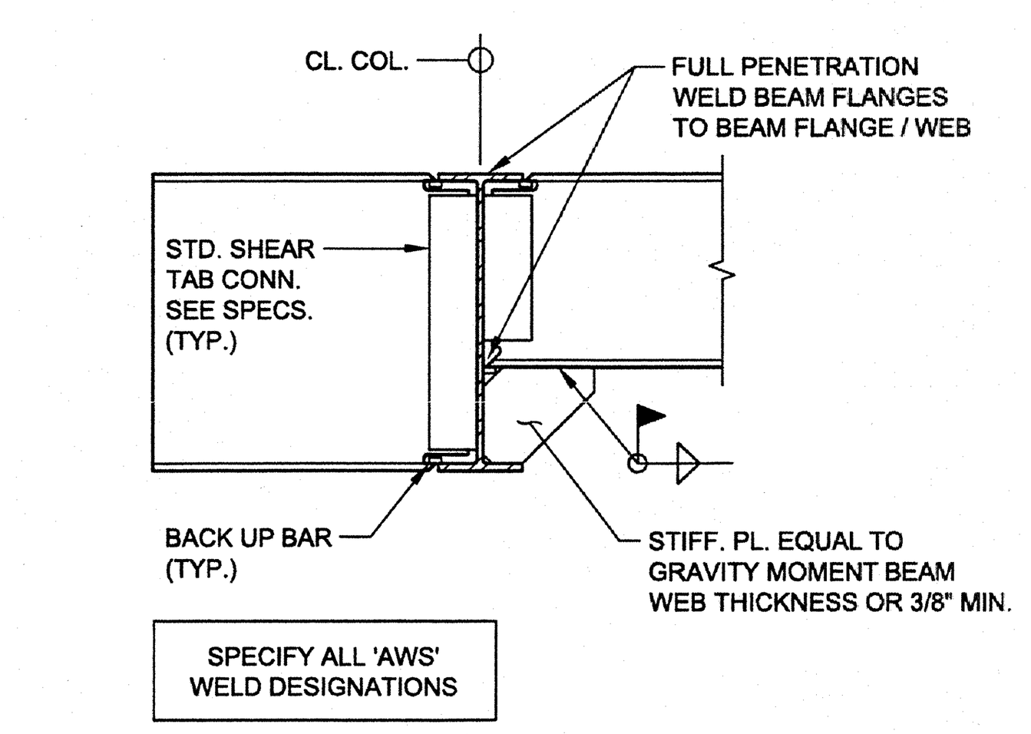


NOTE:
A SPANDREL IS DEFINED AS ANY FRAMING MEMBER WHICH DEFINES THE PERIMETER OF AN AREA OF DISCONTINUITY, I.E. BEAMS AROUND STAIR AND ESCALATOR OPENINGS, ELEVATOR, HVAC, PLUMBING AND ELECTRICAL SHAFTS AND THE PERIMETER OF THE BUILDING.

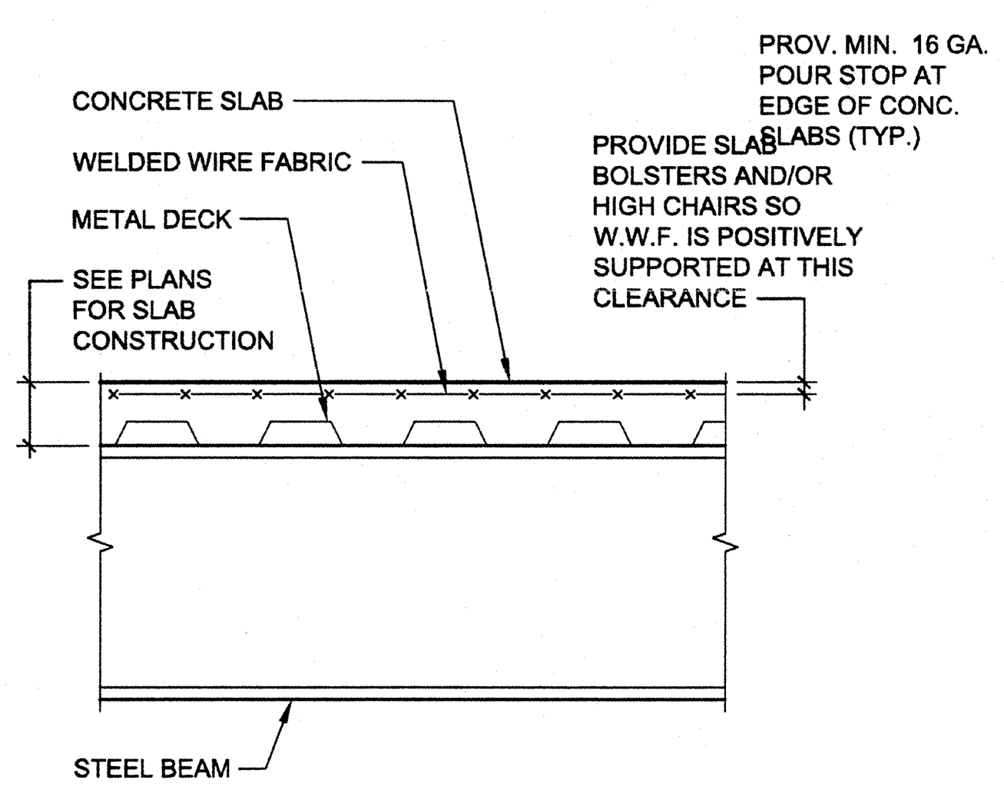
6B TYPICAL DETAIL - FRAMING CONNECTION AT SPANDRELS
NO SCALE SS502



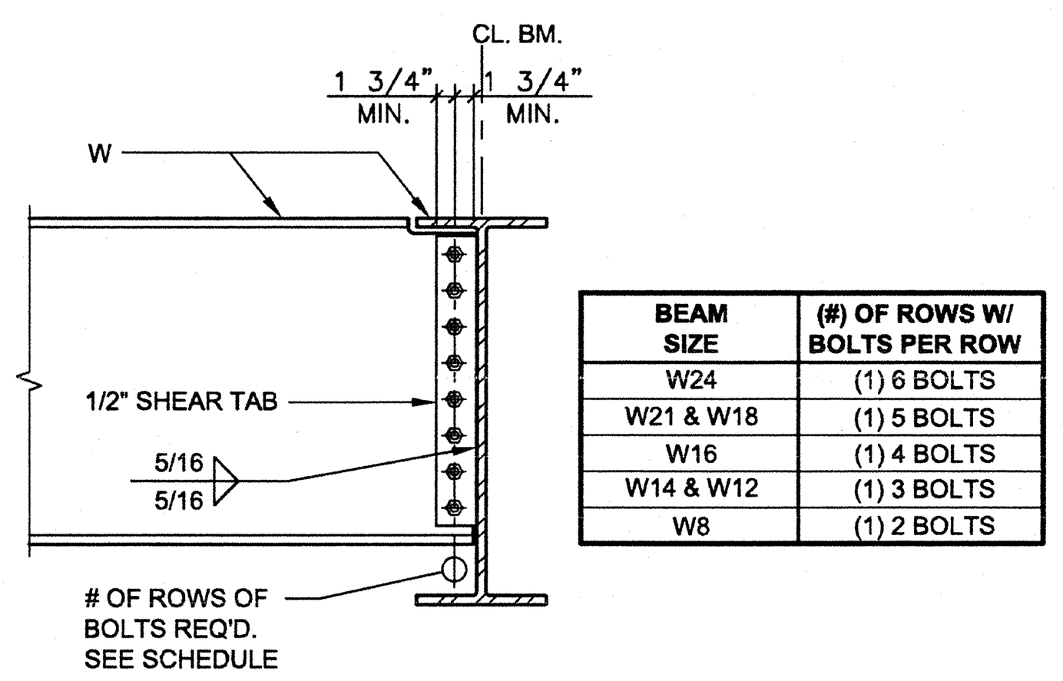
8B TYPICAL DETAIL (M) GRAVITY MOMENT CONNECTION BEAM TO COLUMN WEB
NO SCALE SS502



9B TYPICAL DETAIL (M) GRAVITY MOMENT CONNECTION BEAM TO BEAM
NO SCALE SS502



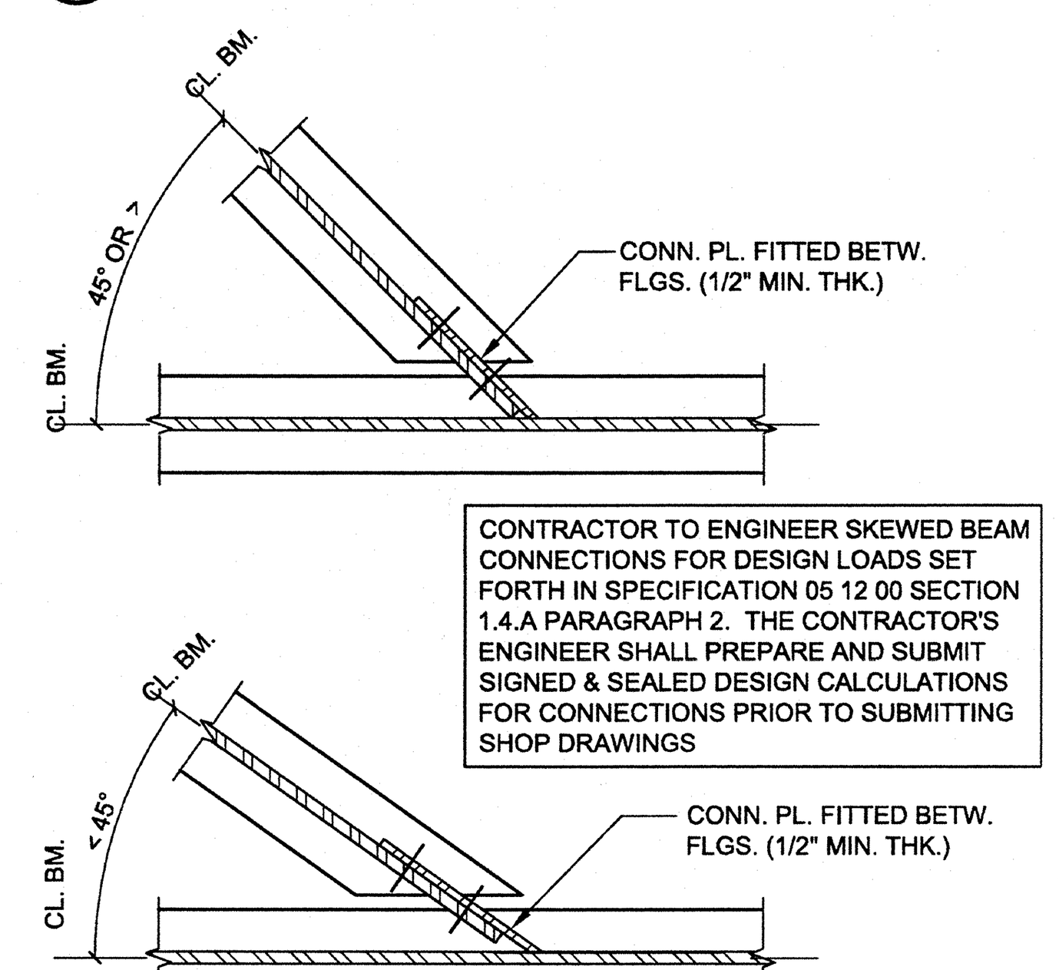
3C TYPICAL DETAIL - SLAB / METAL DECK
NO SCALE SS502



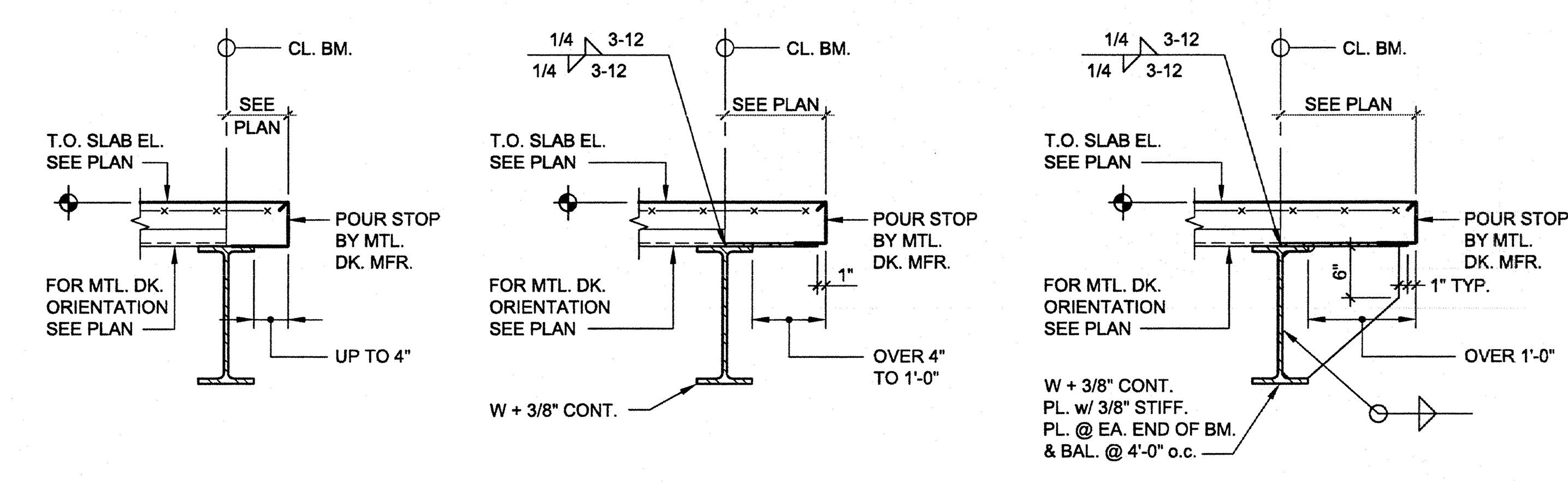
BEAM SIZE	(#) OF ROWS W/ BOLTS PER ROW
W24	(1) 6 BOLTS
W21 & W18	(1) 5 BOLTS
W16	(1) 4 BOLTS
W14 & W12	(1) 3 BOLTS
W8	(1) 2 BOLTS

NOTE:
1. ALL CONN'S. TO BE MADE WITH 7/8"Ø ASTM A490-N HEAVY HEX HEAD BOLTS.
2. CONNECTIONS OTHER THAN STANDARD BOLTED FRAMED CONNECTIONS SHALL BE EQUIVALENT IN CAPACITY TO THOSE LISTED.
3. HIGH STRENGTH HEAVY HEX HEAD BOLTS WILL NOT BE PERMITTED IN THE "SNUG TIGHT" CONDITION. ALL HIGH TENSILE BOLTS MUST BE TORQUED TO SPECIFIED VALUE AS NOTED IN AISC SPECIFICATION FOR STRUCTURAL JOINTS.

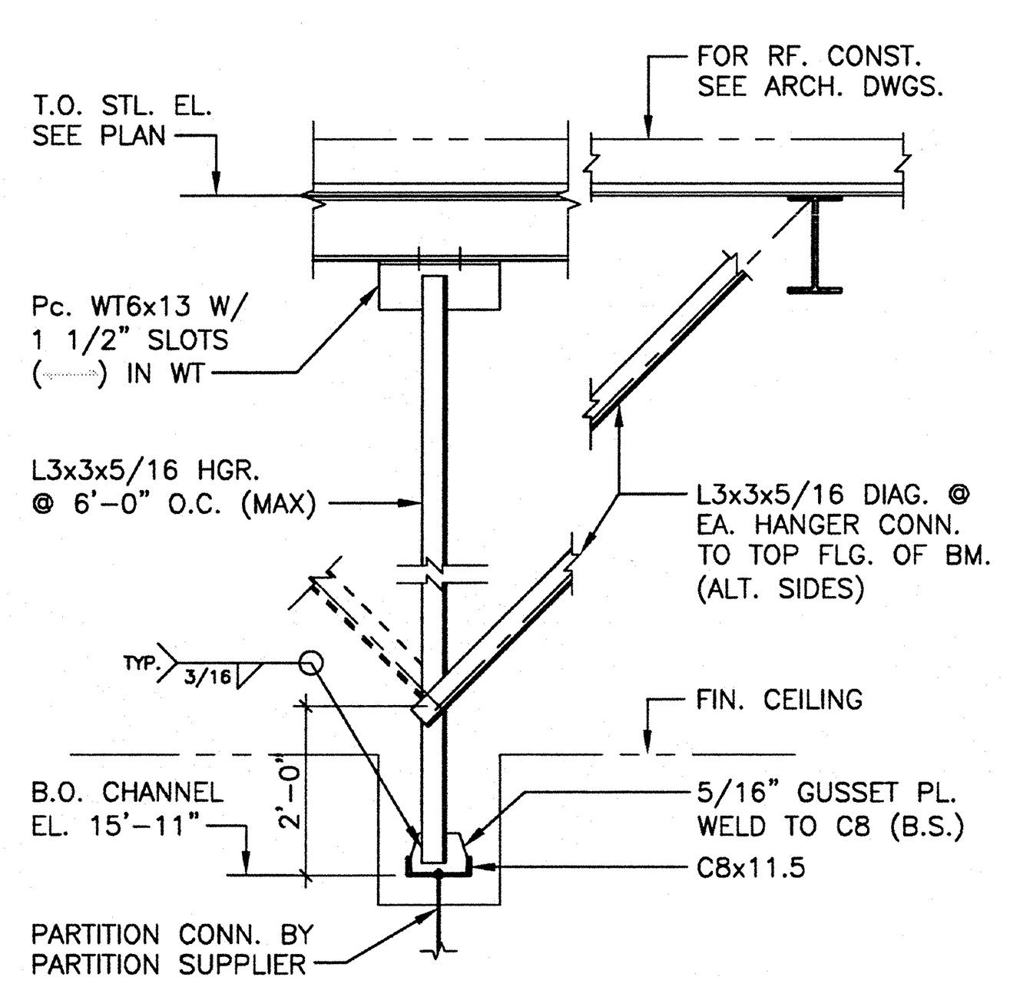
5C TYPICAL DETAIL - CONNECTIONS
NO SCALE SS502



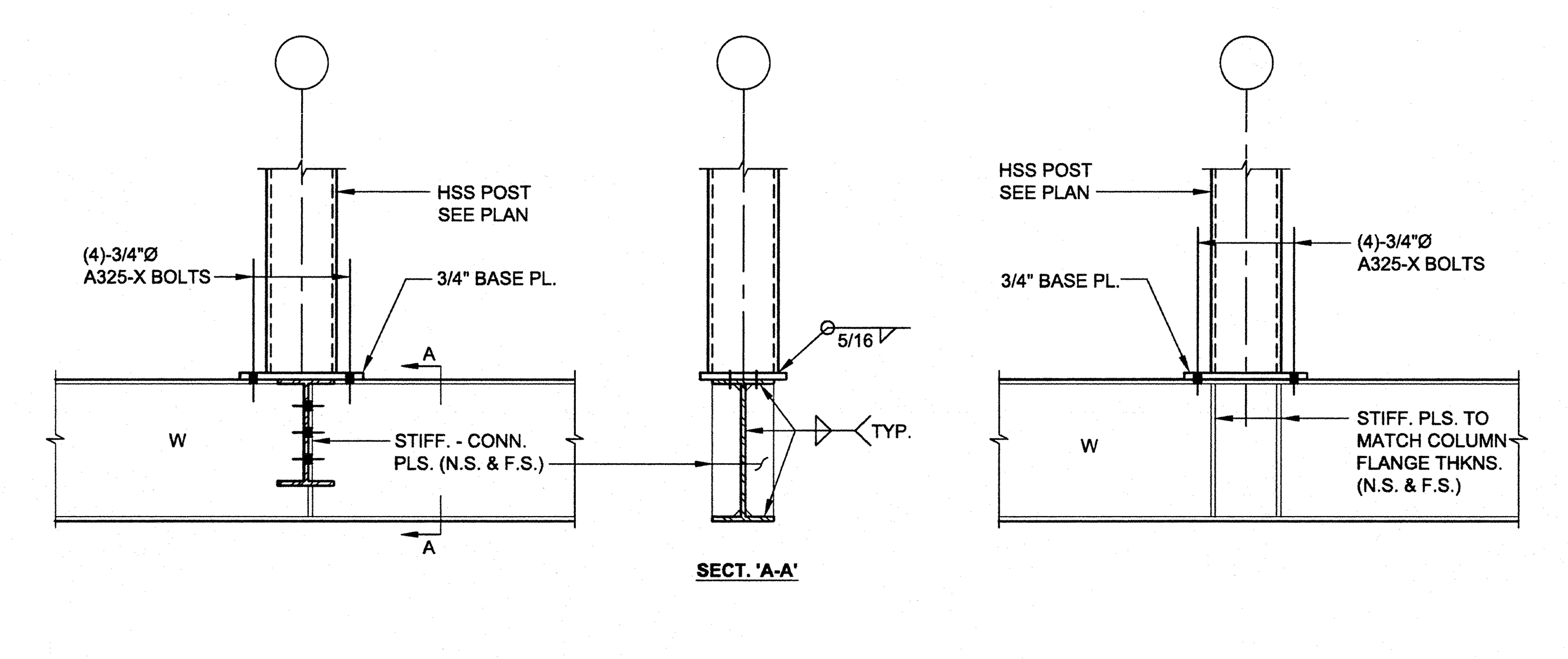
6C TYPICAL DETAIL - SKEWED BEAM TO BEAM CONNECTIONS
NO SCALE SS502



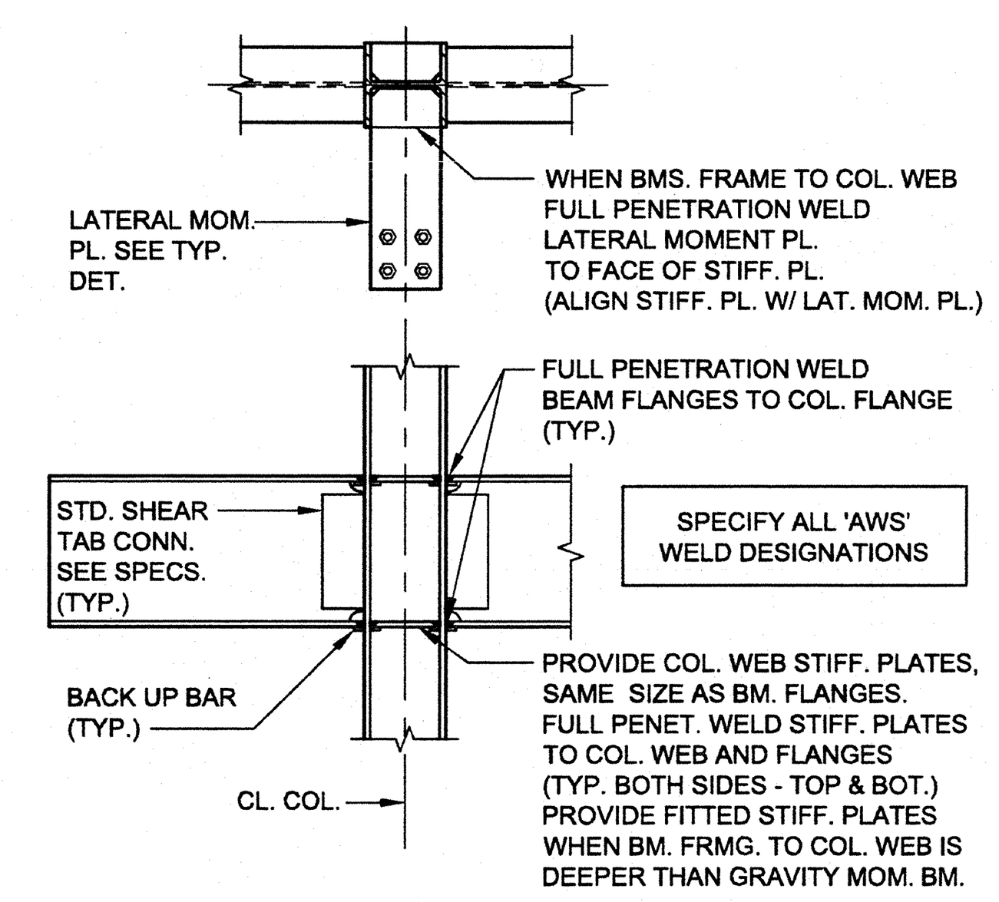
8C TYPICAL DETAIL - FLOOR OPENINGS
NO SCALE SS502



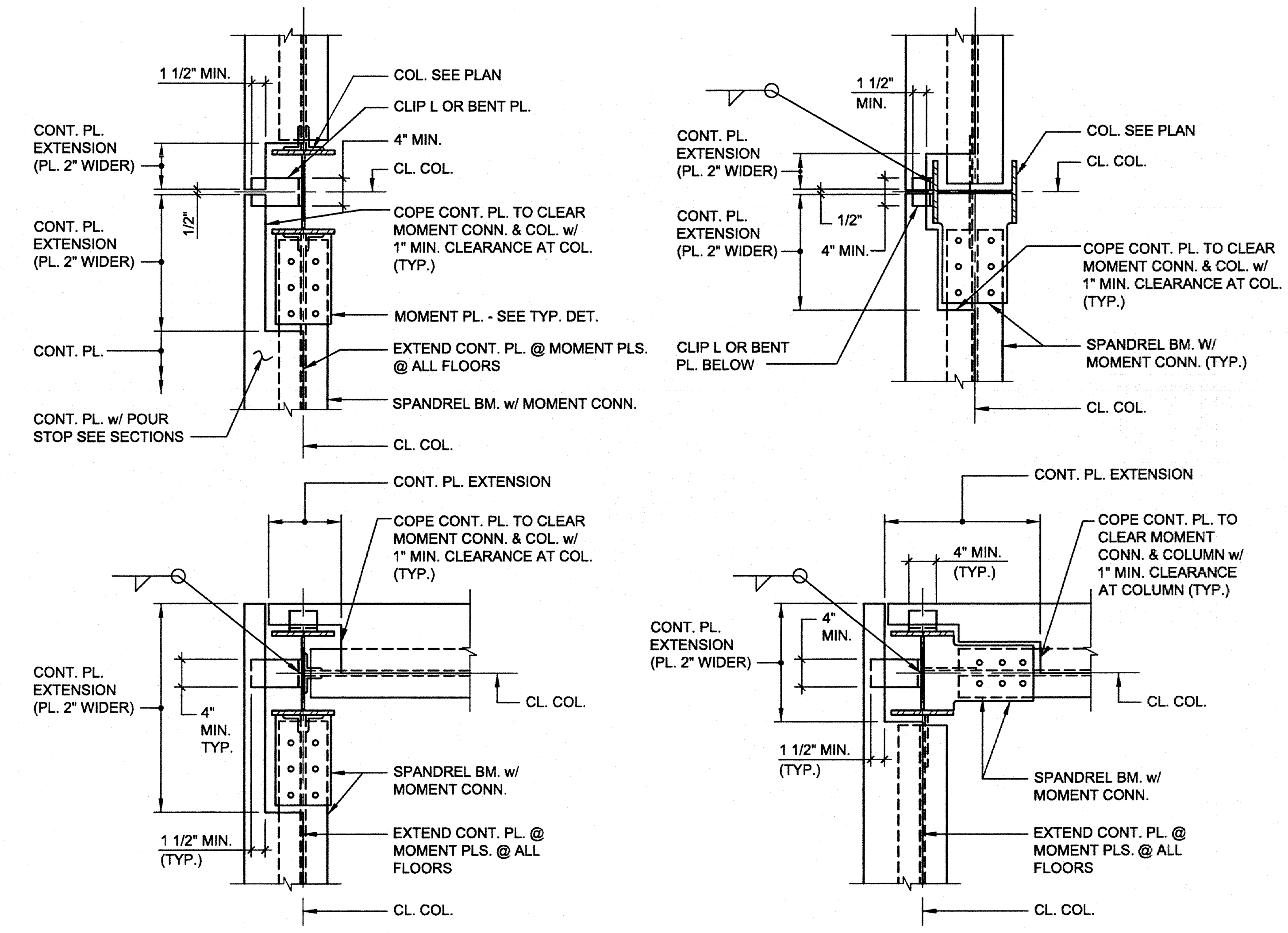
2D TYPICAL DETAIL - HANGING PARTITION SUPPORT
NO SCALE SS502



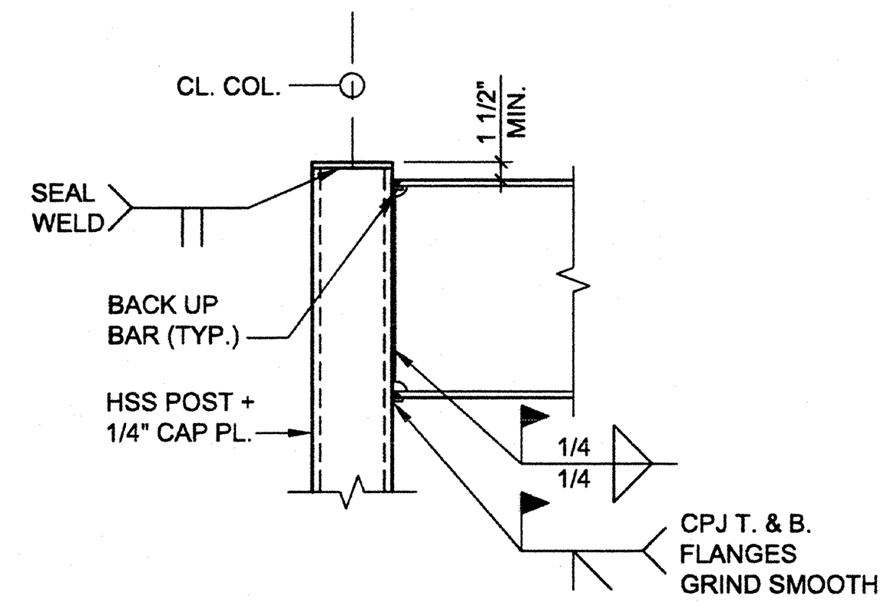
3D TYPICAL DETAIL - POST UP CONNECTION
NO SCALE SS502



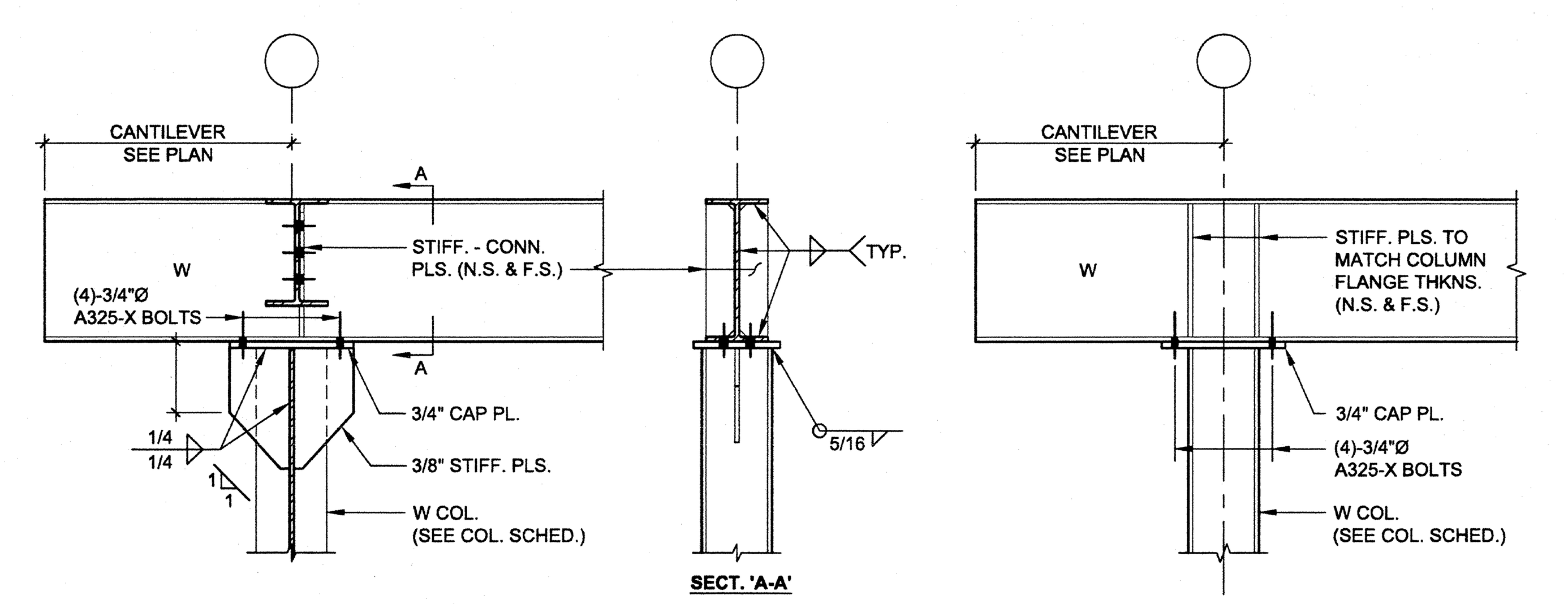
6D TYPICAL DETAIL (M) GRAVITY MOMENT CONNECTION BEAM TO COL. FLANGES
NO SCALE SS502



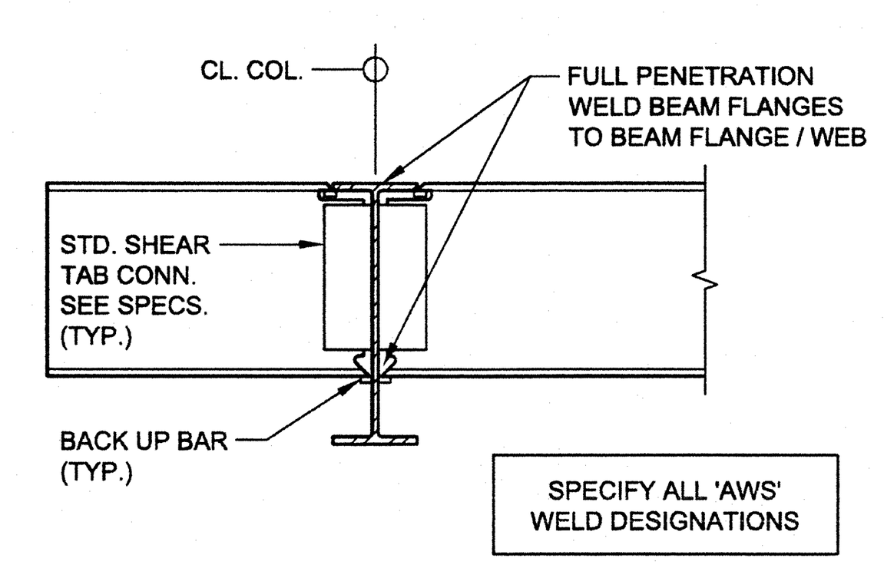
8E TYPICAL DETAIL - CONT. PLATE EXTENSION SUPPORT @ PERIMETER COLUMNS
NO SCALE SS502



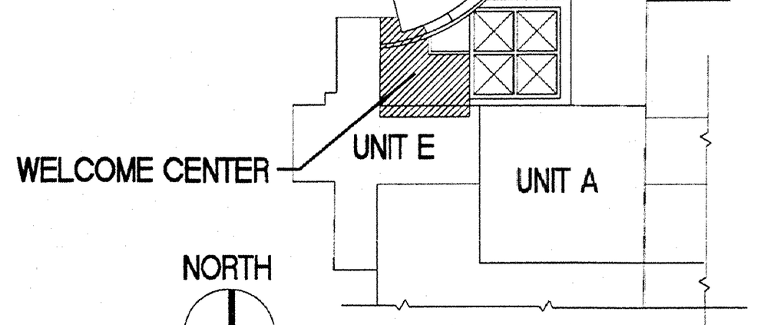
2F TYPICAL DETAIL LATERAL MOMENT CONNECTION @ HSS POST
NO SCALE SS502



3F TYPICAL DETAIL - CANTILEVER BEAM
NO SCALE SS502



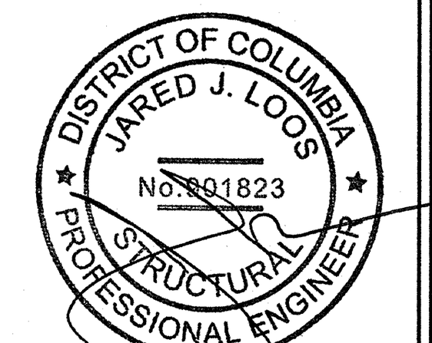
6F TYPICAL DETAIL (M) GRAVITY MOMENT CONNECTION BEAM TO BEAM
NO SCALE SS502



KEY PLAN

Revisions	Date
ISSUE 1 - ISSUE FOR CONSTRUCTION	04.30.2013
95 % SUBMISSION	02.17.2012
75 % SUBMISSION	03.16.2011
25 % SUBMISSION	10.29.2010

CONSULTANTS:



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Drawing Title: TYPICAL DETAILS

Approved: Project Director

Project Title: OIF / OEF WELCOME CENTER DEPARTMENT OF VETERANS AFFAIRS VAMC

Location: Veterans Affairs Medical Center, 50 Irving Street NW Washington DC

Date: 4-30-2013 | Checked: JRM | Drawn: BEC

Project Number: 688-334 OIF/OEF

Building Number

Drawing Number: SS502

Office of Construction and Facilities Management

Department of Veterans Affairs

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